

STRUCTURAL NOTES

GENERAL NOTES

- The governing Code for this Project is the Florida Building Code, 2010 Edition. This Code prescribes which edition of each referenced standard applies to this Project.
- To the best of our knowledge, the Structural Drawings comply with the applicable requirements of the governing Building Code.
- Construction is to comply with the requirements of the governing Building Code and all other applicable Federal, State, and Local Codes, Standards, Regulations and Laws.

DRAWING NOTES

- For complete information use the Structural Documents in conjunction with all other drawings and documents. This includes floor slab slopes, depressions and curbs, types of slab finishes and openings in floors, roofs, walls required by architectural features and MEP systems including ducts, piping, conduit, etc. Openings shown on Structural Drawings are pictorial. See the Architectural and M.E.P. drawings for the size and location of openings in the structure.
- Where member locations are not dimensioned, members are either centered on Grid Lines or are equally spaced between located members.
- Where features or members are not fully shown or specified on the drawings, their construction shall be of the same character as shown or specified in similar conditions.
- Details labeled "typical" apply to all situations that are the same or similar to those specifically referenced, whether or not they are keyed in at each location. Questions regarding the applicability of typical details shall be resolved by the Architect.
- Use of electronic files or reproductions of these contract documents

A. All information contained in the electronic files of the Contract Documents are instruments of service of the Architect/Engineer and shall not be used for other projects, additions to the Project or the completion of the Project by others. Electronic files of the Structural Documents remain the property of Bliss & Nytray, Inc. and in no case shall their transfer be considered a sale.

B. Any contractor, subcontractor, erector, fabricator, material supplier or surveyor signifies their acceptance of all information shown herein as correct, and obligates themselves to any job expense, real or implied, arising due to any errors or omissions that may occur hereon. The use of electronic files does not relieve the Contractor's responsibility for proper checking and coordination of dimensions, details, sizes and quantities.

C. Electronic files are not Contract Documents. In the event of a conflict, the Structural Drawings shall govern.

D. Dimensions and element size and location in the electronic files may not be precise and, in some cases, have been intentionally altered for presentation purposes. Do not scale dimensions electronically or otherwise.

E. When used for preparation of shop drawings, all information not applicable to the subcontract shall be removed from the drawings, including but not limited to, sheet numbers, section marks, title blocks and references to the Contract Documents.

DELEGATED (SPECIALTY) ENGINEER

- A Delegated (Specialty) Engineer is defined as a Florida Licensed Engineer who specializes in and undertakes the design of structural components, systems, shoring, bracing or hoisting for this Project and is an employee or officer of, or consultant to, the Contractor, Subcontractor, Erector or Fabricator. The Delegated (Specialty) Engineer shall sign, seal and date the submittal, including calculations and drawings.
- Submittals for special structural, load-bearing items that are required by Codes or Standards to resist forces must be prepared by, or under the direct supervision of a Delegated (Specialty) Engineer. Examples include prefabricated wood components, structural light gage steel framing, exterior enclosure systems and shoring, reshoring and bracing.

CONTRACTOR REQUIREMENTS

- The term "contractor" refers to either General Contractor, Construction Manager or the entity the Owner has assigned to have overall supervision and responsibility for the work.
- The Contractor shall supervise and direct all work and shall be responsible for construction means, methods, procedures, techniques, and sequence. The Contractor has sole responsibility for the quality and correctness of the work. The Contractor shall protect adjacent property, his own work and the public from harm. The Contractor shall be responsible for jobsite safety, procedures and programs, including all OSHA requirements.
- The Contractor shall ensure that all construction methods used will not damage adjacent buildings and property and is advised to perform all photographic surveys and other documentation of the adjacent buildings and property prior to and during construction.
- Contractors who discover discrepancies, omissions or variations in the Contract Documents during bidding shall immediately notify the Architect. The Architect will resolve the condition and issue a written clarification.
- The Contractor shall coordinate all Contract Documents with field conditions and dimensions and Project Shop Drawings prior to construction. Do not scale drawings; use only printed dimensions. Report any discrepancies in writing to the Architect prior to proceeding with work. Do not change size or location of structural members without written instructions from the Structural Engineer of Record.
- The Contractor shall review and approve all submittals, including all substitution requests and shop drawings, and shall sign and date each drawing prior to submitting to the Architect. This approval is to confirm the submittal is complete, complies with the submittal requirements and is coordinated with field dimensions, other trades, erection sequencing and constructability. Submittals not approved by the Contractor will be returned by the Structural Engineer without review.
- The Structure is designed to resist the code required vertical and lateral loads and to be structurally sound when completed. The Contractor shall not overload the structure during construction. Prior to completion, the Contractor is responsible for stability and temporary bracing, including, but not limited to, masonry walls. Wherever the Contractor is unsure of these requirements, the Contractor shall retain a Specialty Engineer to design and inspect the temporary bracing and stability of the structure.

STRUCTURAL ENGINEER OF RECORD'S (EOR) ROLE DURING CONSTRUCTION

- Bliss & Nytray, Inc. (BNI) shall not have control or charge of the work, the quality of the work and does not have the authority to stop work or make changes to the contract sum. BNI shall not have control or charge of safety precautions and programs. BNI shall not be responsible for the acts or omissions of the contractor, subcontractor, or any other entity or person performing the work, or with the failure of same to carry out the work in accordance with the Contract Documents.
- BNI reviews submittals to confirm that the submittal is in general conformance with the design concept presented in the Construction Documents. Quantities and dimensions are not checked. Notations on submittals do not authorize changes to the contract sum. Checking of the submittal by BNI shall not relieve the Contractor of responsibility for deviations from the Contract Documents and from errors or omissions in the submittal.
- BNI's review of submittals prepared by a Delegated (Specialty) Engineer is limited to verifying that the specified submittal has been furnished, signed and sealed by the Delegated (Specialty) Engineer and that the Delegated (Specialty) Engineer understood the design intent and used the specified structural criteria. No detailed check of the calculations is made. The Delegated (Specialty) Engineer is solely responsible for his/her design, including but not limited to the accuracy of his/her calculations and compliance with the applicable codes and standards.
- Periodic site visits by BNI is for the purpose of becoming generally familiar with the progress and quality of the work and determining, in general, if the work observed is being performed in a manner, that when fully completed will be in general accordance with the Structural Contract Documents. Periodic site visits do not provide extensive and continuous check of the quality, accuracy or quantity of the Contractor's work.

DESIGN SUPERIMPOSED LOADS

1. GRAVITY LOADS

OCCUPANCY	LIVE LOAD	DEAD LOAD
Roof	20 psf	15 psf

2. WIND LOADS

Governing Code	ASCE 7-10
Building Risk Category	II
Ultimate Wind Speed	V = 175 mph (3 second gust)
Allowable Stress Design Wind Speed	V = 138 mph (3 second gust)
Mean Roof Height	12 feet
Directionality Factor	K _d = 0.85
Topographic Factor	K _z = 1.0
Gust-Effects Factor	G = 0.85
Exposure -W/WFS	C
Components and Cladding	C
Enclosure Classification	Enclosed
Internal Pressure Coefficient	G _{Cpi} = 0.18

ENCLOSURE PROTECTION FROM WIND BORN DEBRIS

This Building is located in a (High Velocity Hurricane Zone and) Wind Born Debris Region and has been designed as enclosed. All parts of the building envelope, including but not limited to, cladding systems, exterior doors and windows, skylights, glass block shall meet impact test criteria or be protected with an external protection devise that has been tested and meets the impact test criteria in accordance with Section 1626 of the Florida Building Code and shall have a current Notice of Acceptance (NOA).

SHOP DRAWINGS AND OTHER SUBMITTALS

- Submit specific components, such as columns, footings, etc., in a single package.
- On first submittal, clearly flag and cloud all differences from the Contract Documents. On resubmittals, flag and cloud all changes and additions to previous submittal; only clouded items will be reviewed.
- The Trade Contractor is responsible for confirming and correlating dimensions at the job sites, for tolerances, clearances, quantities, fabrication processes and techniques of construction, coordination of the work with other trades and full compliance with the Contract Documents.
- The Contractor shall submit for EOR review shop drawings for the following items:
 - Reinforcing steel
 - Embedded items or items attached to the structural frame for building cladding attachment or for attachment of other items (Prepared by Delegated Engineer)
 - Construction joint locations in beams, walls, and slabs on grade
 - Wood trusses and truss system
- The Contractor shall submit for review by the noted entity and a copy of the approved drawing to the EOR for their records, the following items:
 - Concrete mix designs (Submit to Testing Agency for approval) (For EOR records only)
 - Concrete masonry unit material product sheets and mix designs (Submit to Testing Agency for approval) (For EOR records only)
 - Shoring (Prepared by Specialty Engineer) (For EOR records only)

SHALLOW FOUNDATIONS

- Foundation design, Soil preparation and compaction are based on Geotechnical Investigation, Data and Recommendations in Report B-131165 by Neko Testing and Engineering Services Inc. dated November 15th 2013.
- Footing sizes and reinforcing are based on an allowable soil bearing capacity of 2000-psf. All footings shall bear in compacted fill or natural soil prepared per the Geotechnical Report.
- Subgrade preparation shall be field controlled and tested by a Licensed Sole Engineer in accordance with the Geotechnical Report. At completion, that Engineer shall prepare and submit to the owner, Architect, Contractor and Structural Engineer a signed and sealed letter indicating that the recommendations of the Geotechnical Report have been followed.
- Center all footings under their respective walls, u.o.n.
- Top of all footings is 1'-4" below the ground floor level, u.o.n.

EXCAVATION, BACKFILL AND DE WATERING

- The Contractor is solely responsible for all excavation procedures including lagging, shoring, and protection of adjacent property, structures, streets and utilities in accordance with the requirements of the local building department and OSHA regulations. Do not excavate within one foot of the angle of repose of any soil bearing foundation unless the foundation is properly protected against settlement.
- The Contractor is responsible for the disposal of all accumulated water in a manner that does not inconvenience or damage the work.

SLABS ON GRADE

- Above subgrade, use fill containing not more than 10% passing #20 sieve and maximum 1 inch diameter. Compact to 95% of maximum dry density as determined by modified proctor ASTM D-1557. Each layer of fill shall not exceed 6" loose thickness. Compact prior to placement of the next layer.
- Fill placement and compaction shall be monitored and accepted by the testing agency. Take a min. of one field density test (ASTM D-1556 or D-2922) for each 2,500 square feet of each layer. The testing agency shall randomly select test locations.
- For interior slabs place 10 mil polyethylene sheeting between soil and bottom of slab. Do not use any sheeting below exterior concrete slabs.
- Use 4" thick slabs on grade reinforced with 6 x 6 - W2.9 welded wire reinforcement supplied in flat sheets only. Use chairs to support wire fabric in the center of slab.
- Place concrete in long-strip construction method. Provide crack control joints at 16 feet maximum to limit areas between joints to 256 sq. ft. in all floating slabs on grade. Locate to conform to bay spacing whenever possible, add crack control joints at re-entrant corners which tend to invite cracks.
- In sidewalks and walkways, locate isolation joints at 20 ft. o.c. maximum score and tool between isolation joints in equal bays of 5 ft. or less.
- See the Architectural Drawings for slab on grade depressions and other requirements.

REINFORCED CONCRETE

- Comply with ACI 301 and 318.
- Provide structural concrete with a minimum ultimate compressive design strength in 28 days as follows:

Element	Strength
Footings	3000 psi
Columns	4000 psi
Slabs on Grade	3500 psi

- Use normal weight concrete for all structural members. u.o.n.

- Provide ASTM A-615 Grade 60 reinforcing steel. Reinforcing shall be accurately placed, rigidly supported and firmly tied in place, with appropriate bar supports and spacers. Lap continuous reinforcing 48 bar dia. Provide cover over reinforcing as follows:

Element	Bottom	Top	Sides
Footings	3"	2"	3"
Beams Above Grade	1 1/2"	1 1/2"	1 1/2"
Columns	-	-	1 1/2"
Slabs on Grade	2"	1"	2"

- Where specified, provide plain, cold-drawn electrically-welded wire reinforcement conforming to ASTM A-185. Supply in flat sheets only. Lap splice one cross wire spacing plus two inches.
- Where reinforcing steel congestion permits, conduit and pipes up to 1" diameter may be embedded in concrete per ACI 318, Section 6.3. Space at 3 diameters o.c. Place between outer layers of reinforcing. If conduits are significantly congested, additional reinforcing perpendicular to piping may be required. Requests to embed larger pipes should be accompanied by a detailed description and be submitted to the architect for evaluation.
- Provide construction joints in accordance with ACI 318, Section 6.4. Provide keyways and adequate dowels. Submit drawings showing location of construction joints and direction of pour for review.
- Provide 3/4" chamfer for all exposed corners.
- Provide reinforcing steel placer with a set of Structural Drawings for field reference. Inspect reinforcing steel placing from structural drawings.

CONCRETE MASONRY

- Construct masonry in accordance with ACI 530/ASCE 5, "Building Code Requirements for Concrete Masonry Structures"; and ACI 530.1/ASCE 6, "Specifications for the Design and Construction of Load-Bearing Concrete Masonry".
- The structure is supported by bearing walls. Erect masonry prior to casting concrete columns within bearing walls or casting beams and slabs supported by bearing walls.
- Use 50% solid, nominal 8x8x16, concrete masonry units conforming to ASTM C90. Block net area compressive strength shall be 1900 psi. Lay up units in running bond. Sawcut units which are not in multiples of 8". Units shall be at least 8" long. Bond corners by lapping ends 8" in successive vertical courses. Design of walls is based on a f_m of 1500 psi.
- Use Type 5 mortar in accordance with ASTM C270 except use Type M mortar below grade. Head and bed joints shall be 3/8" for the thickness of the face shell. Webs are to be fully mortared in all courses of piers, columns and pilasters; in the starting course; and where an adjacent cell is to be grouted. Remove mortar protrusions extending 1/2" or more into cells to be grouted.
- Use standard (9 gauge) horizontal joint reinforcing in every other course. Joint reinforcing and anchors in exterior walls shall conform to ASTM A 153 Class B2, with a coating thickness of 1.50 oz/sf; conform to ASTM A 641 in interior walls. Overlap discontinuous ends 6". Use prefabricated corners and tees. Use truss type, except use ladder type in walls with vertical reinforcing. Extend joint reinforcing a minimum of 4" into tie columns.
- Use fine grout conforming to ASTM C-476, with a minimum compressive strength of 2500 psi in 28 days. Aggregate to conform to ASTM C404 for fine grout, with slump of 8" to 10". Grout all masonry containing reinforcing, all cells of 4 hour rated walls, and where indicated on the drawings. Allow mortar to cure 24 hours prior to grouting. Provide cleanout openings at the base of cells containing reinforcing steel to clean the cell and to tie the vertical bar to the dowel. In high-lift grouting, use 5'-0" (max.) lifts, with 1/2 hour to 1 hour between lifts. Vibrate each lift and reconsolidate the previous lift.
- Use ASTM A-615 Grade 60 reinforcing steel. Reinforce walls where indicated on the drawings and at all intersections, each side of openings and at the ends of walls. Use bar spacers at 10 ft. o.c. where grout pour height exceeds 10 ft..
- At bond/tee beam corners and intersections, place 1 #5 x 5'-0" T & B corner bar, with 30" legs each way, at the exterior face.
- Beams not scheduled are min. 8" x 12" tie beams with 2 #5 bars top and bottom and #3 ties spaced at 48" o.c. typical and 4 ties at 12" o.c. at ends and intersections, u.o.n. columns not scheduled are min. 8" x 12" tie columns with 4 #5 vertical bars and #2 ties at 12" o.c. use 30" lap splices. Hook all bars at discontinuous ends.
- Reinforced masonry wall construction shall be inspected by an Engineer or Architect in accordance with ACI 530.1/ASCE 6.
- Where anchor bolts, wedge anchors or anchors set in epoxy are set in a masonry wall, fill cells with grout for bolted course, one course above and two courses below.
- Provide lintels or headers with min. 8" bearing over all masonry openings.
- Use pressure-treated wood for wood in contact with masonry.

POWDER ACTUATED FASTENERS

- Use powder actuated fastening systems such as those manufactured by Hilti, ITW Rammed Red Head, Powers, or an accepted alternate having ICC-ES or Miami-Dade County Product Control Approval. Install in accordance with Manufacturer's Specifications. Provide a minimum of two fasteners per connection.
- Provide a minimum penetration in accordance with manufacturer's specifications but not less than 1-1/8" in concrete, u.o.n.
- Provide a minimum anchor spacing and edge distance of 3" in concrete and a minimum anchor spacing of 1" and edge distance of 1/2" in steel.

PRE-ENGINEERED WOOD TRUSSES

- Design and fabricate all timber in conformance with the "National Design Specification for Stress-Grade Lumber and Its Fastenings" and Florida Building Code, 2010 Edition, and the National Design Standard for metal plate connected wood truss construction.
- In accordance with Rule 61G15-31.003 of the Florida Administrative Code, the Truss Design Engineer, a Delegated Engineer, shall design the individual trusses of the Truss System, but does not design the Truss System. The Truss Design Engineer shall submit shop (piece) drawings and calculations for each different truss of the Truss System such that each truss will function to support the dead, live and wind loads applicable to each truss and truss girder that together comprise the Truss System. These shop drawings and calculations shall be signed and sealed by the Truss Design Engineer.
- The loads, layouts and connections provided on the structural construction documents are the minimums to be followed by the Truss Design Engineer.
- Use stress-rated timber for all wood structural members. Do not use wood structural members with a bending stress less than 1200 psi or a modulus of elasticity less than 1,400,000 psi when used at 15% maximum moisture content.
- Pressure treatment of all structural lumber shall be in accordance with AWPA Standards C1 and C2, latest Editions with a waterborne preservative in accordance with Standard P5, latest Edition. All lumber to be kiln-dried after treatment to a moisture content not to exceed 19% over-dry basis, per Standard C2. All lumber 4 x 4 in. (nominal size) and larger in dimension to be treated to the soil and fresh water retention and penetration requirements of Standard C2. All lumber less than 4 x 4 (nominal size) to be treated to the above ground requirements of Standard C2.
- Minimum design loads for roof trusses:
20 psf LL top chord

- 15 psf DL top chord
- 10 psf DL bottom chord
- Wind pressure based on ASCE-7. See General Notes and Drawing S-1.1
- The design and erection of wood trusses, including bracing, shall conform to the commentary and recommendations of the truss plate institute. Provide lateral bracing (minimum 2" thick nominal lumber) as follows:

A. In the plane of the bottom chord (perpendicular to the trusses) provide continuous bottom chord bracing as indicated on Drawing S-2.0.

B. In the plane of the web members (perpendicular to the trusses): at each web member requiring continuous lateral bracing as indicated on the Truss Design Engineer's Drawings, but at a spacing not more than 20'.

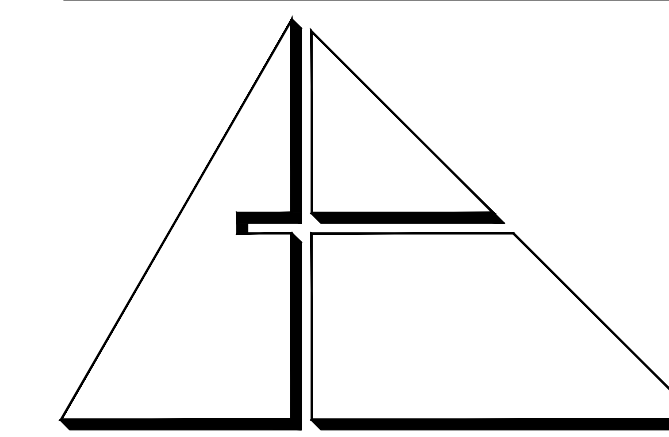
- Anchor all diagonal bracing to reinforced masonry walls or reinforced concrete members with prefabricated (min. 12 ga.) galvanized steel straps or framing connectors. Fasten straps to masonry with 2-1/2" dia. masonry anchors or 4-1/2" dia. x 1-1/2" powder-driven pins if into concrete and to wood members with not less than 6-16D nails.

- Plywood roof sheathing shall be 5/8" thick C-D interior with exterior glue APA. In compliance with AITC Standards, connect to supports with 10d ring shank nails at 6" o.c. at panel edges and intermediate supports and 4" o.c. at roof edges and shear walls. Place face grain perpendicular to supports. Provide 1/16" space at end joints and 1/8" at edge joints. Provide pycilps along edge joints at mid span between supports.

POST-INSTALLED ANCHORS

- Post-installed anchors may be used where any post-installed or mechanical anchors are called for on the Contract Documents. All post-installed anchors shall comply with ACI 318-02, Appendix D and be tested and qualified under the provisions of ACI 355.2 and ICC ES AC 193. Anchors are to be manufactured from carbon steel (in corrosion prone environments specify stainless steel) and shall have an electroplated or mechanically galvanized finish.
- In un-cracked concrete (compression zones) the Contractor may request a substitute anchor for approval so long as he can substitute that it will only be used in un-cracked concrete locations.
- Contractor shall obtain approval from Structural Engineer of Record (SER) prior to using post-installed anchors for missing or misplaced cast-in-place anchors.
- Confirm the absence of reinforcing steel by drilling a 1/4" diameter pilot hole for each anchor. Do not cut reinforcing steel without approval of the SER.
- Anchors shall be installed per the manufacturer's installation instructions at not less than minimum edge distances and/or spacings indicated in the manufacturer's literature.
- Unless specified otherwise, anchors shall be embedded in the appropriate substrate with a minimum embedment of 8 times the nominal anchor diameter or the embedment required to support the intended load.
- Substitution requests, for products other than those listed below, shall be submitted to the SER with calculations that are prepared & sealed by a licensed Professional Engineer showing that the substituted product will achieve an equivalent capacity using the appropriate design procedure required by the Building Code.

Acceptable manufacturers:
a. Simpson Strong-Tie Co. Inc.
b. Hilti, Inc.
c. Powers Fasteners, Inc.
d. Williams Form Engineering, Corp.



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PROJECT: BNI # 12M18.02

CITY OF MIAMI
LIFE QUEST
DEMOLITION
AND
REPLACEMENT
14950 NE 11TH AVENUE

MIAMI, FL

DRAWING TITLE:

STRUCTURAL
NOTES

SCALE

George N. Khoury, P.E. Fla. Reg. No. 55479

BNI Project No.: 12M18.02

DATE:

August 12, 2013

REVISED:

November 20, 2013 Building Department

CAD DRAWING NAME:

SHEET NO.

BID SET

S-1.0

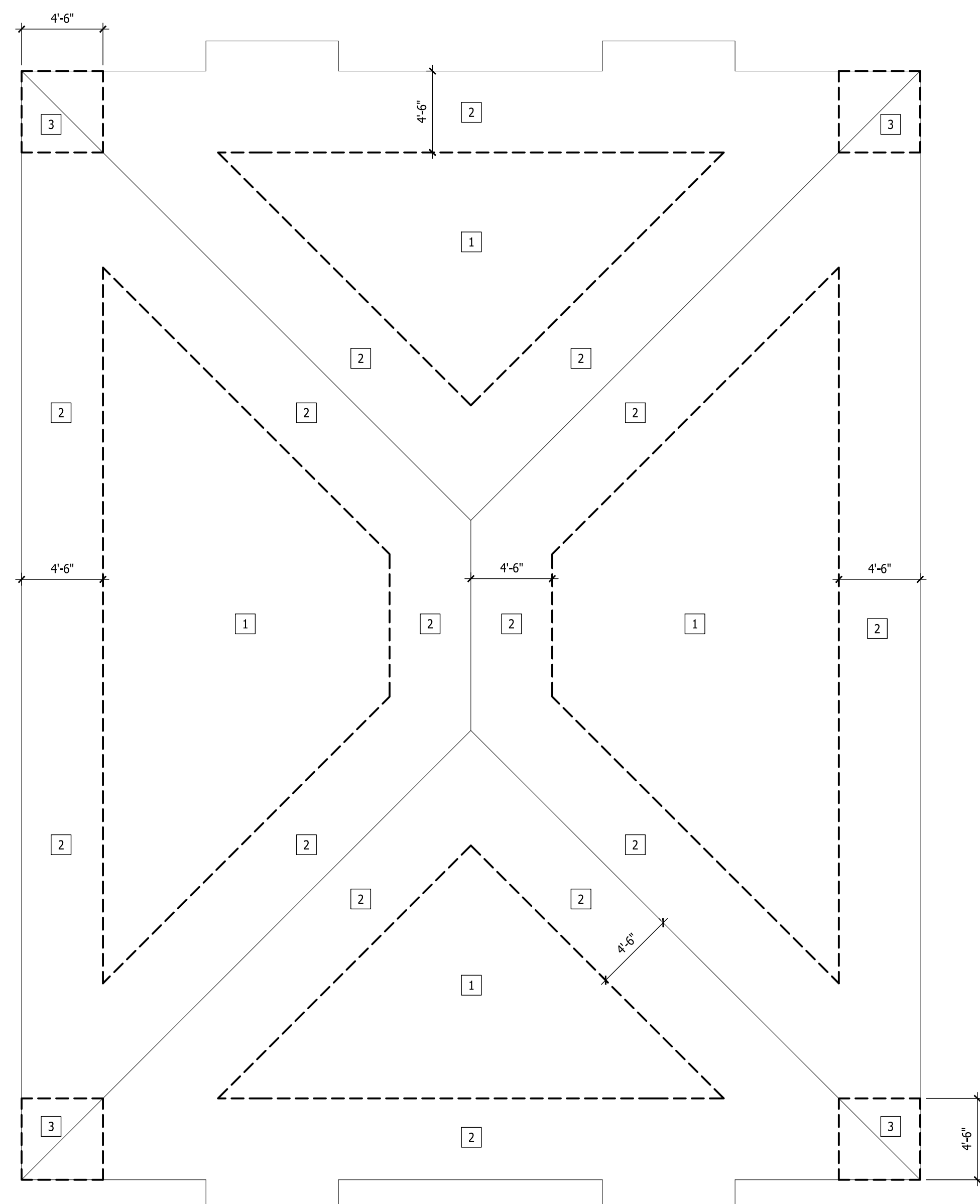
COMPONENTS AND CLADDING ROOF OVERHANG PRESSURES (PSF)						
ZONE	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
2	-81	-81	-81	-81	-81	-81
3	-132	-120	-104	-91	-91	-91

COMPONENTS AND CLADDING ROOF PRESSURES (PSF)						
ZONE	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
1	24/-37	22/-36	19/-35	17/-34	17/-34	17/-34
2	24/-64	22/-59	19/-52	17/-47	17/-47	17/-47
3	24/-95	22/-89	19/-81	17/-74	17/-74	17/-74

COMPONENTS AND CLADDING WALL PRESSURES (PSF)						
ZONE	TRIBUTARY AREA (SF)					
	10	20	50	100	200	500
4	41/-44	39/-42	36/-40	35/-38	33/-36	30/-34
5	41/-54	39/-51	36/-46	35/-42	33/-39	30/-34

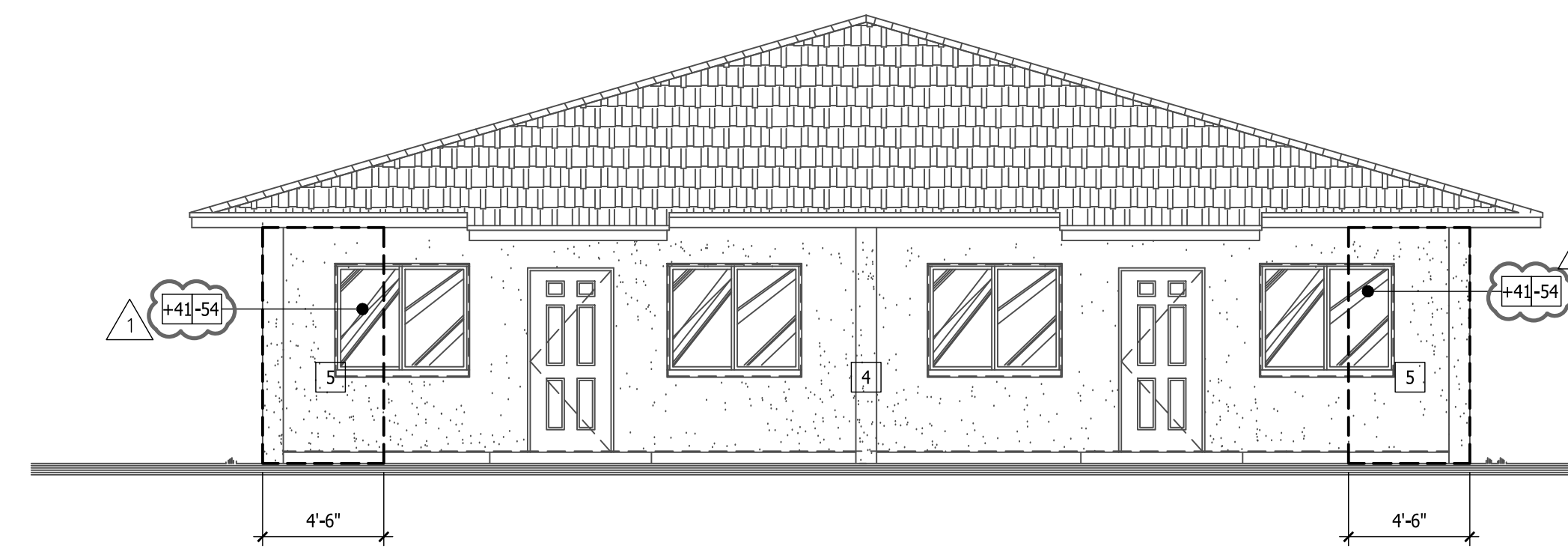
WIND PRESSURE NOTES

- Numbers on this sheet are the components and cladding gross unfactored service pressures perpendicular to the surface (in P.S.F.) based on tributary area. Multiply service pressures by 1.6 to obtain wind pressures for factored loads using strength design (ASCE 7-10 2.3).
- Pressures are derived from ASCE 7-10.
- Directionality factor $K_d = .85$
- Negative pressures act away from surface, positive pressures act toward surface.
- All dimensions shown are measured perpendicular to surface.
- Overhang pressures apply to components which observe wind from both the roof and soffit, such as cantilevered roof truss ends.
- All doors and windows shall have a design wind pressures of $+11/-14$ psf, unless otherwise noted by $+2/-4$.

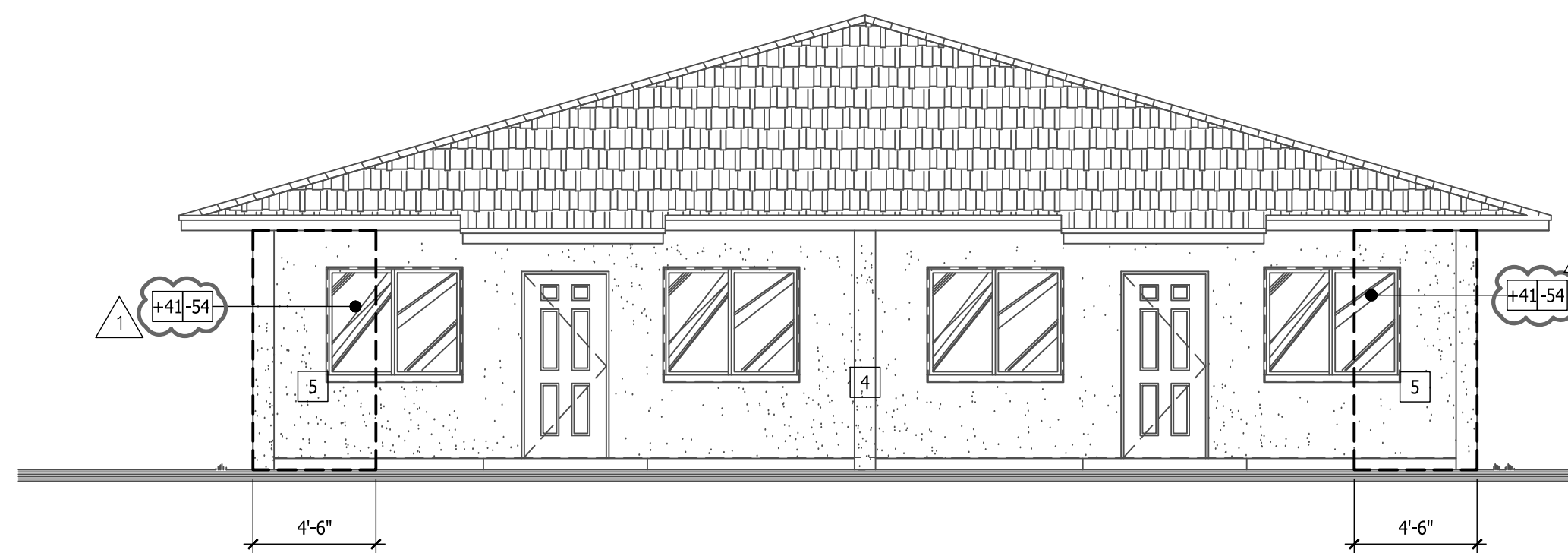


PLAN VIEW

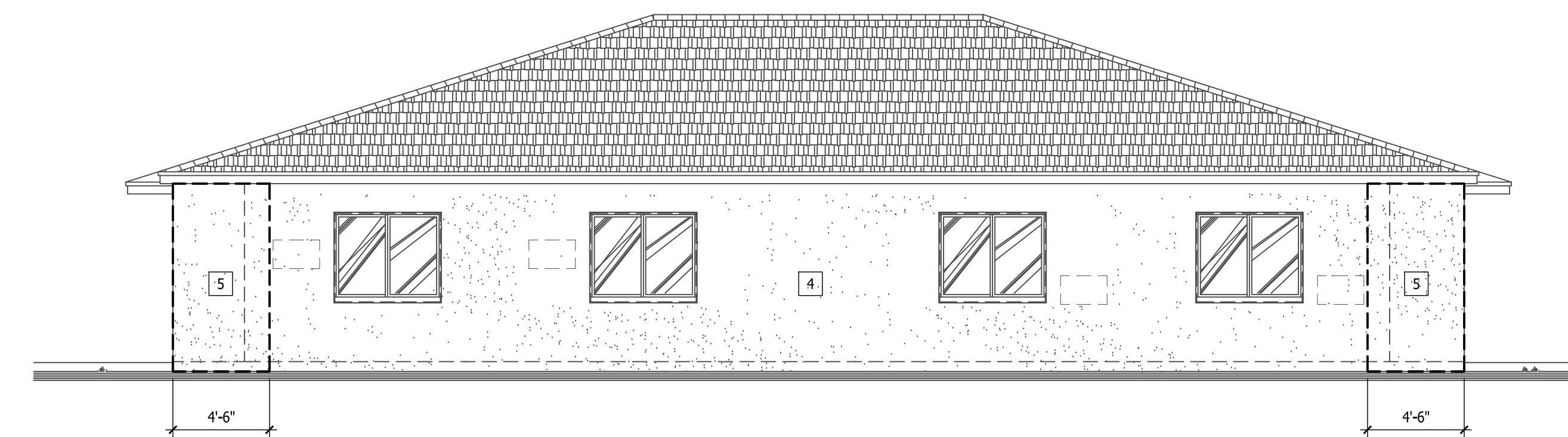
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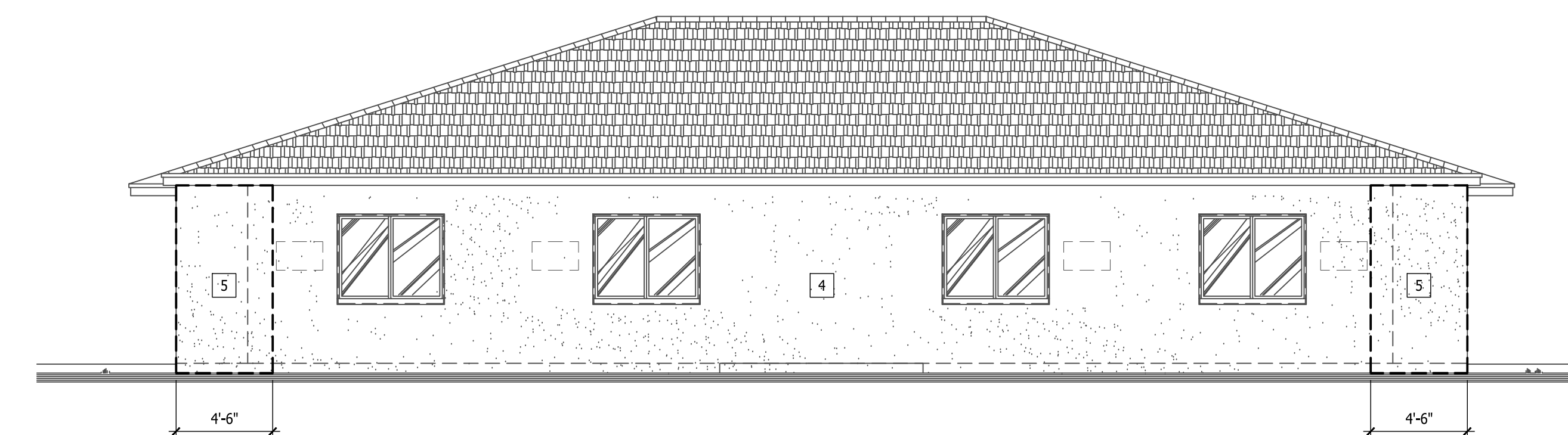
1 NORTH ELEVATION



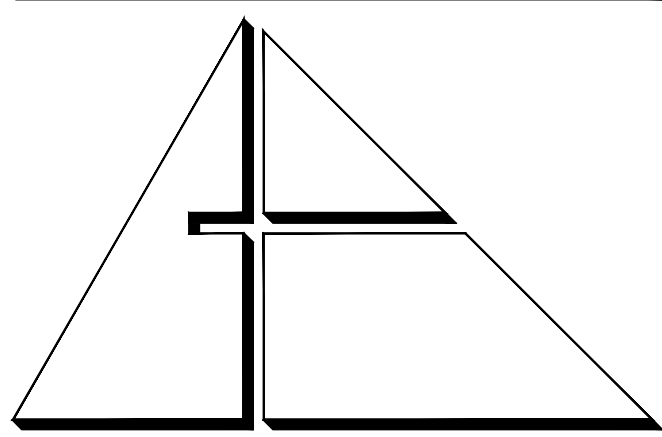
2 SOUTH ELEVATION



3 EAST ELEVATION



4 WEST ELEVATION



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PROJECT: BNI # 12M18.02

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 DEMOLITION
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 REPLACEMENT
 14950 NE 11TH AVENUE

MIAMI, FL

DRAWING TITLE:
**WIND PRESSURE
 DIAGRAMS**

SEAL

George N. Khoury, P.E. Fla. Reg. No. 55479
 BNI Project No.: 12M18.02

DATE:
 August 12, 2013

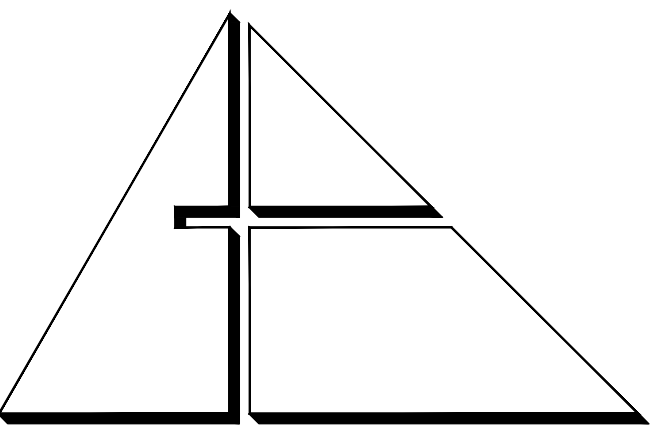
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CITY OF MIAMI
LIFE QUEST
DEMOLITION
AND
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14950 NE 11TH AVENUE

MIAMI, FL

DRAWING TITLE:

FRAMING PLANS

SEAL

George N. Khoury, P.E. Fla. Reg. No. 55479

BNI Project No.: 12M18.02

DATE:

August 12, 2013

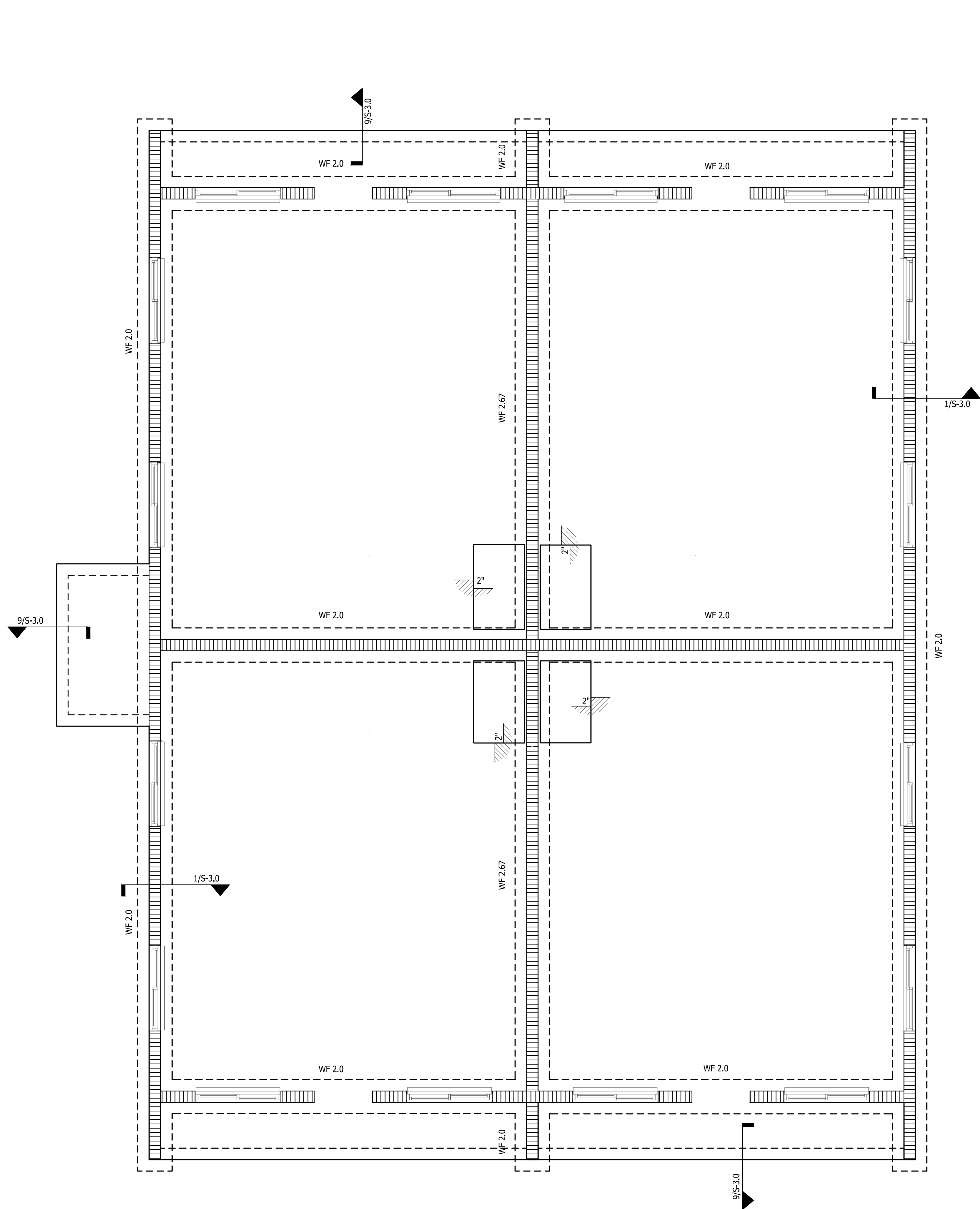
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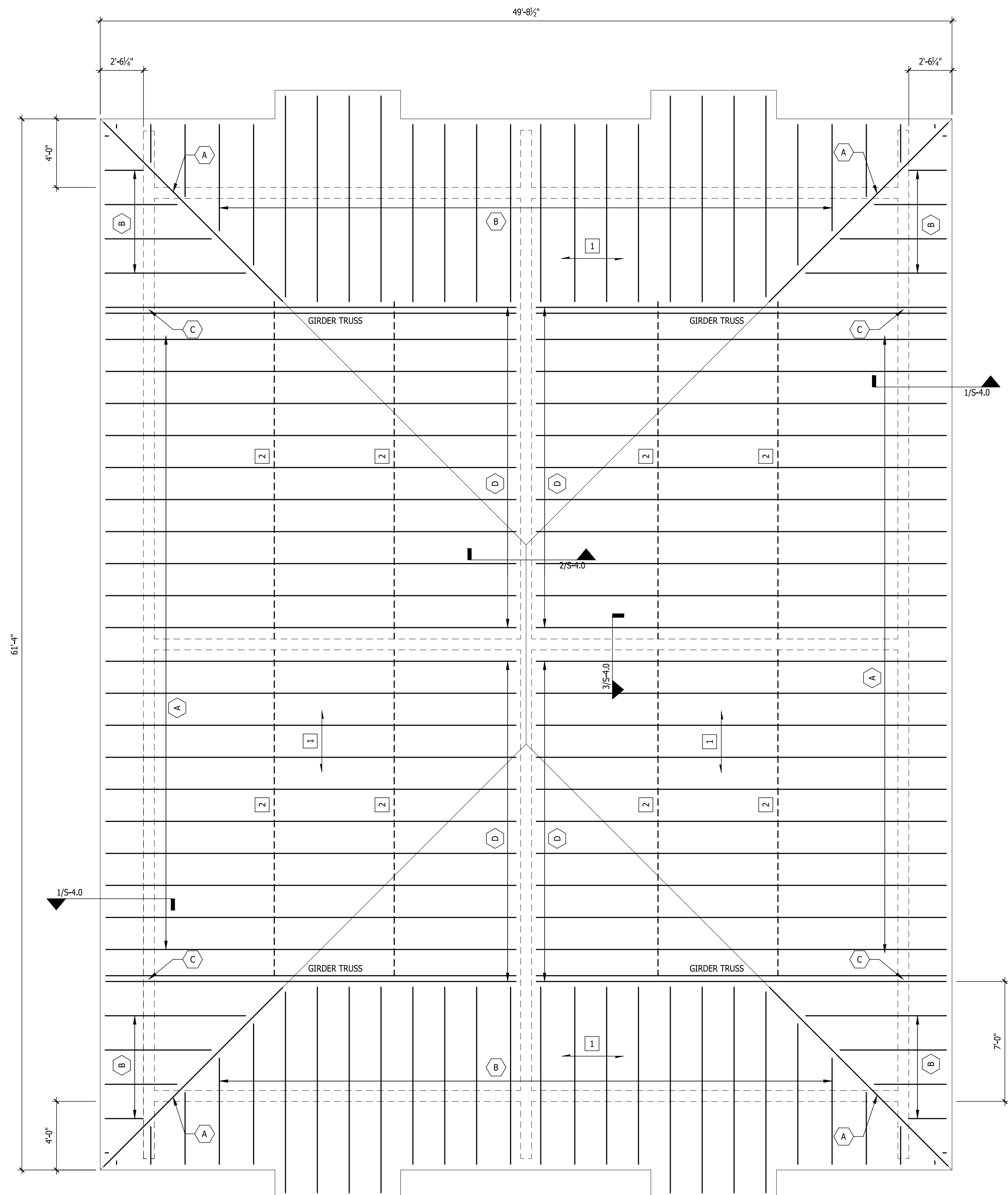
S-2.0



A FOUNDATION/SLAB ON GRADE PLAN
SCALE: 1/4"=1'-0"
N

GROUND FLOOR PLAN NOTES:

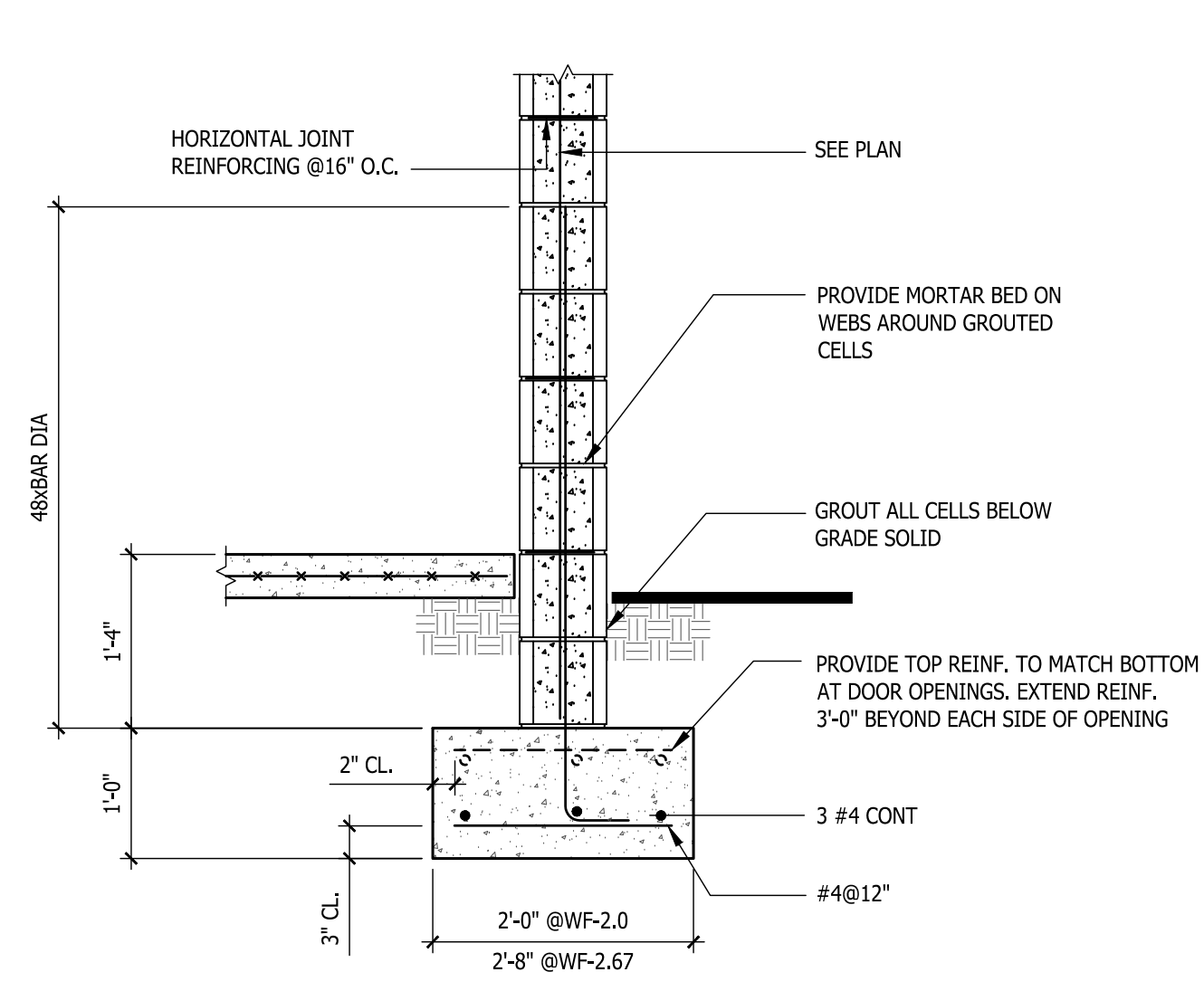
- 1 TOP OF FOOTING IS 1'-8" BELOW GRADE U.N.O.
- 2 SEE STRUCTURAL NOTES FOR SLAB ON GRADE.
- 3 REINFORCE ALL MASONRY WALL WITH #5 @32" O.C.
- 4 SEE ARCH FOR PLAN DIMENSIONS



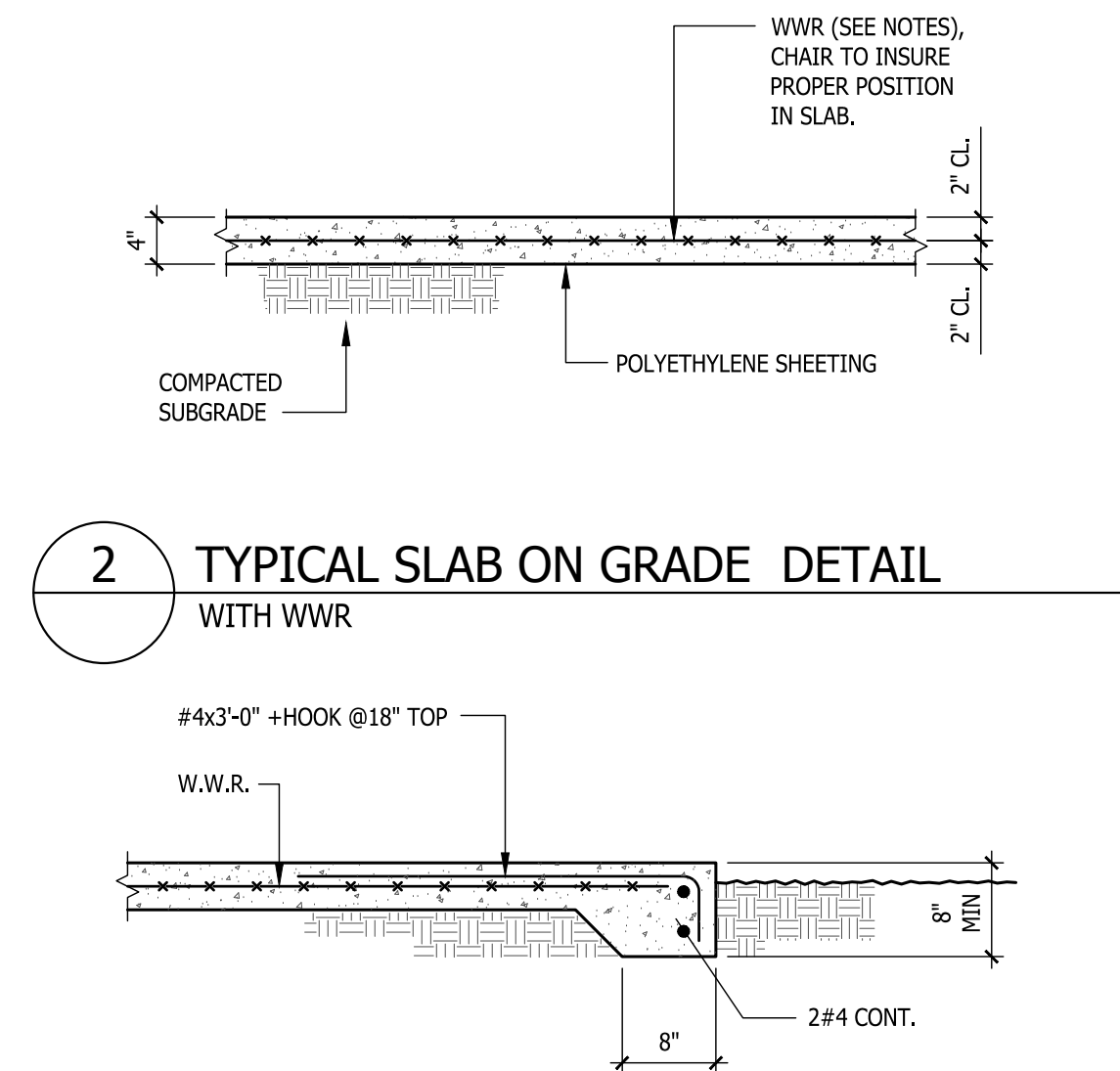
B ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"
N

ROOF PLAN NOTES:

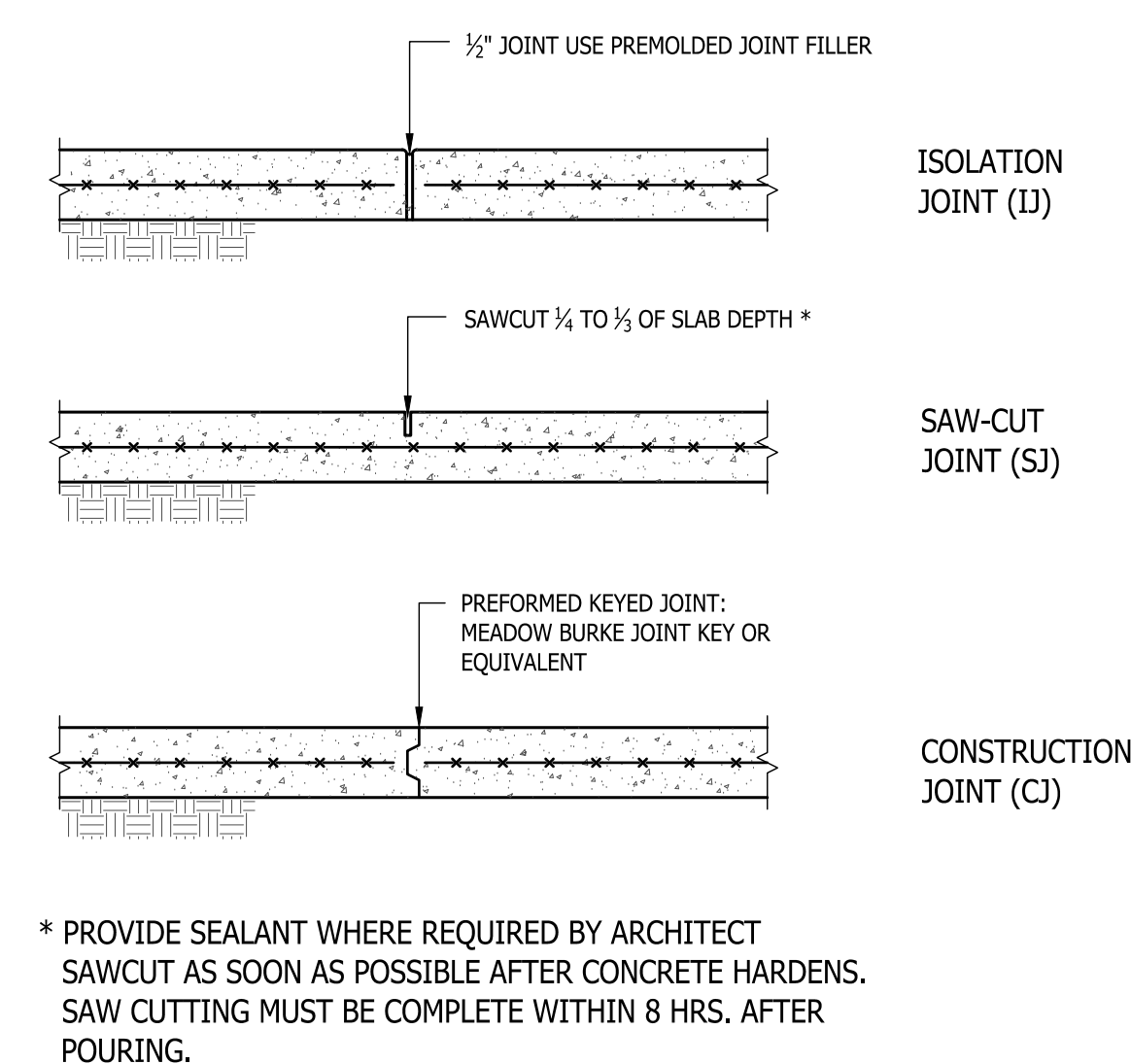
- 1 ROOFING (SEE ARCH.) OVER 3/4" PLYWOOD SHEATHING OVER PRE-ENGINEERED WOOD TRUSSES (BY TRUSS DESIGN ENGINEER) @2'-0" O.C. MAX. SEE STRUCTURAL NOTES FOR FASTENING REQUIREMENTS.
- 2 2X BOTTOM CHORD BRIDGING EQUALLY SPACED. REFER TO STRUCTURAL NOTES FOR ADDITIONAL TRUSS BRIDGING REQUIREMENTS.
- 3 INDICATES SIMPSON TIE DOWN STRAP TO BE USED, SEE SCHEDULE ON SHEET S-4.0
- 4 ALL MASONRY WALLS TO HAVE MIN. 8x16 TIE BEAMS AT TOP, REINFORCE TIE BEAMS WITH 2 #6 CONT. TOP AND BOTTOM AND #4 TIES @48" O.C. TYP. AND 4 #4 TIES @12" AT ENDS.



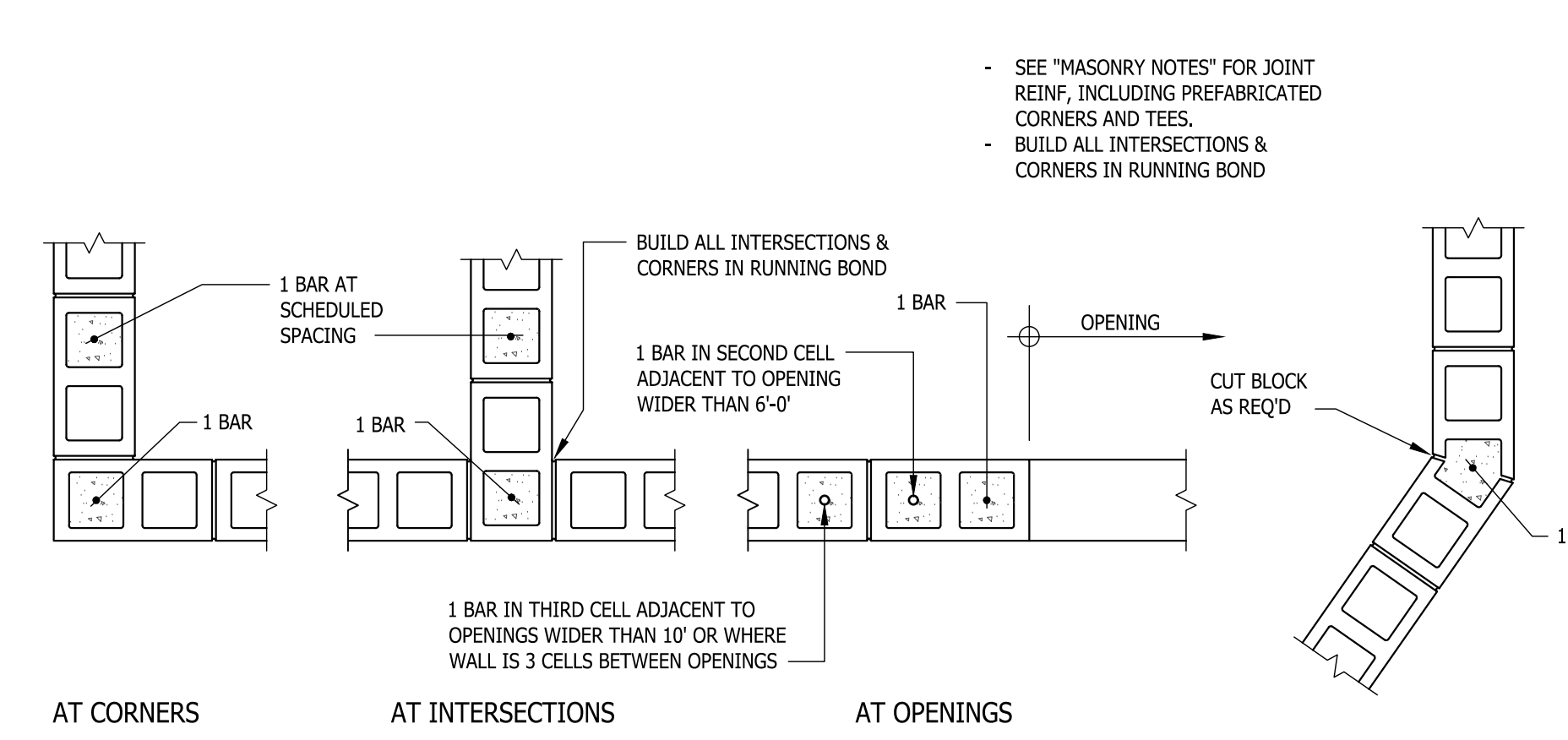
1 WALL FOOTING DETAIL



2 TYPICAL SLAB ON GRADE DETAIL WITH WWR

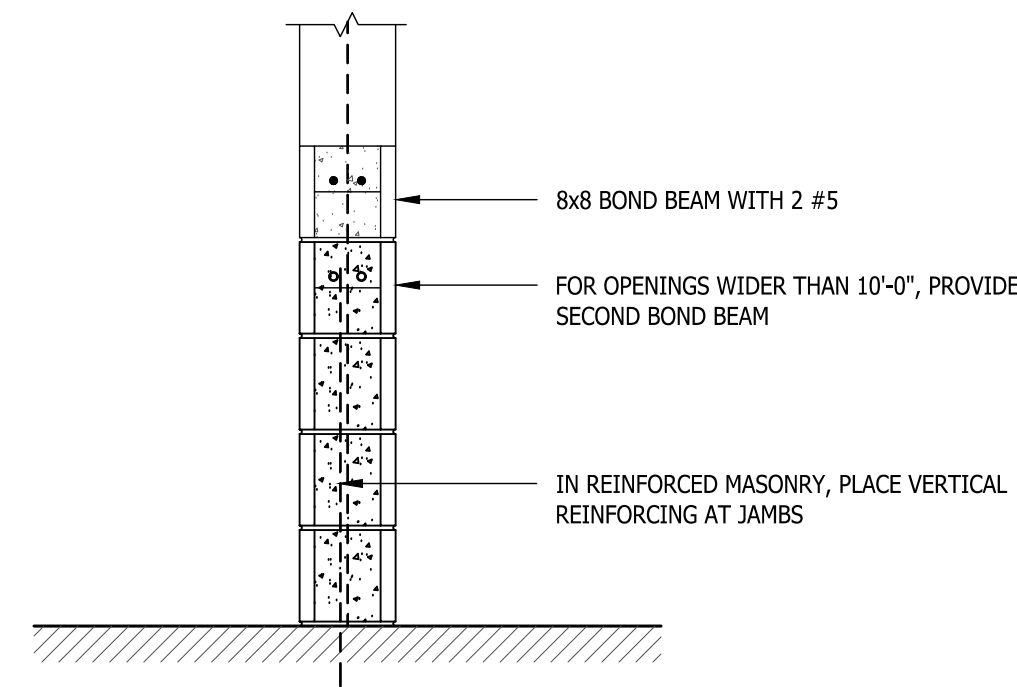


4 CONTROL JOINTS SLAB ON GRADE

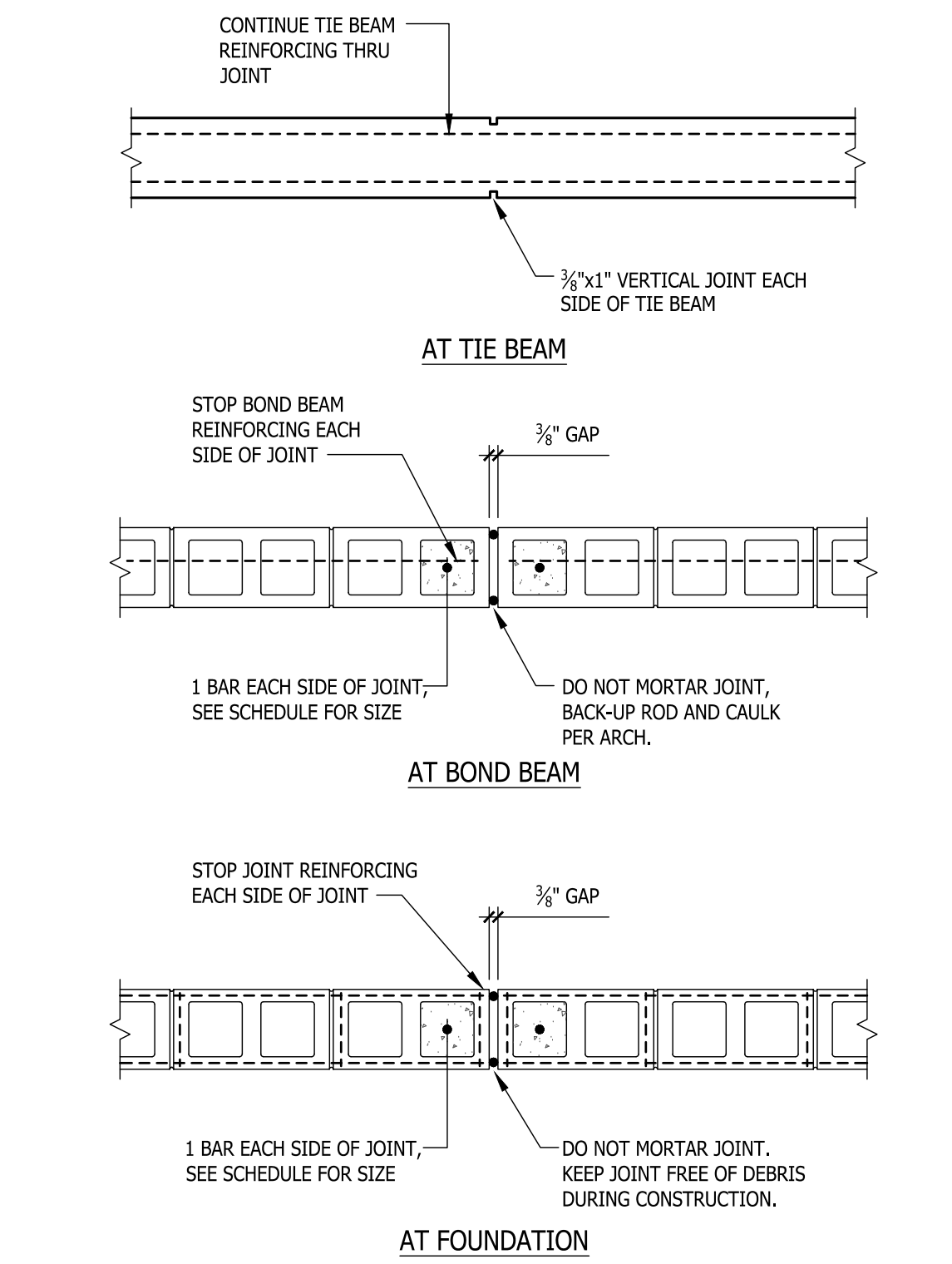


5 MASONRY WALL REINFORCING

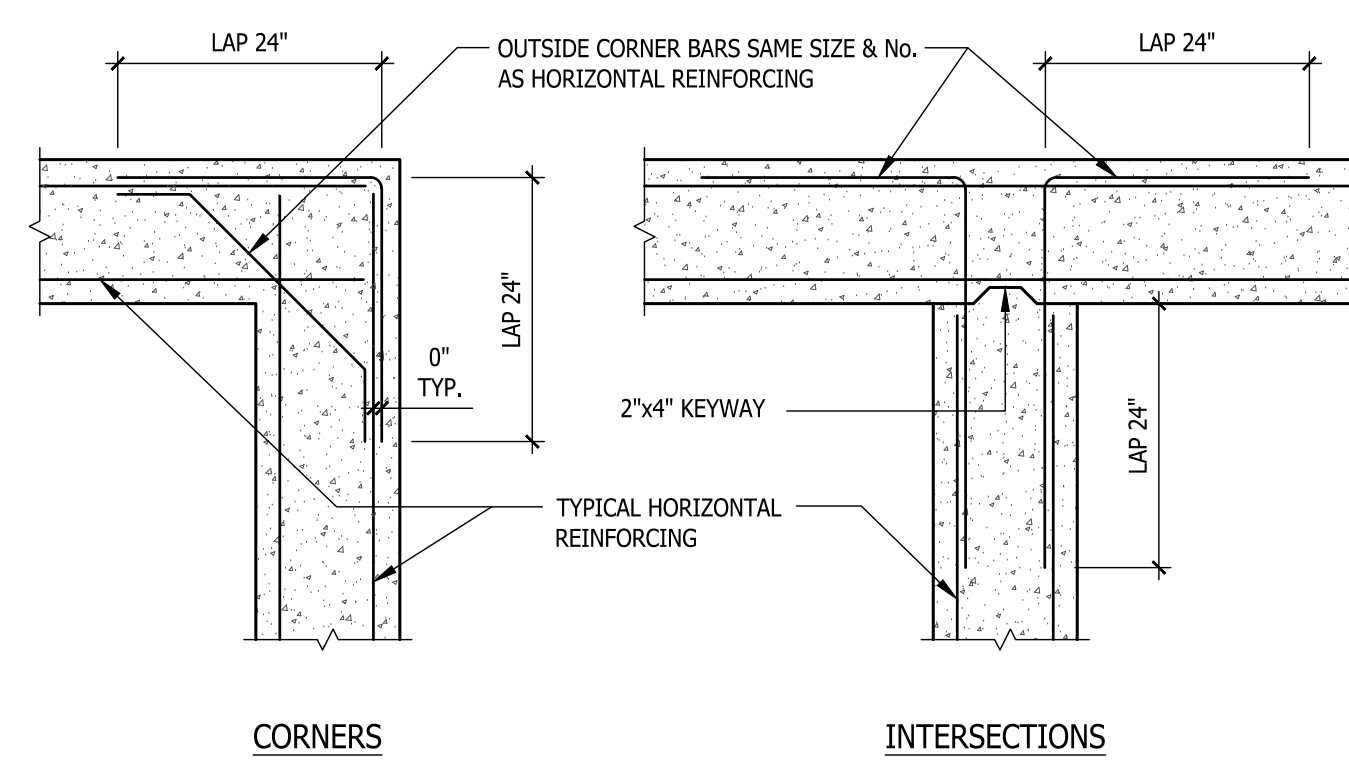
SEE "MASONRY NOTES" FOR JOINT REINF. INCLUDING PREFABRICATED CORNERS AND TEES.
BUILD ALL INTERSECTIONS & CORNERS IN RUNNING BOND



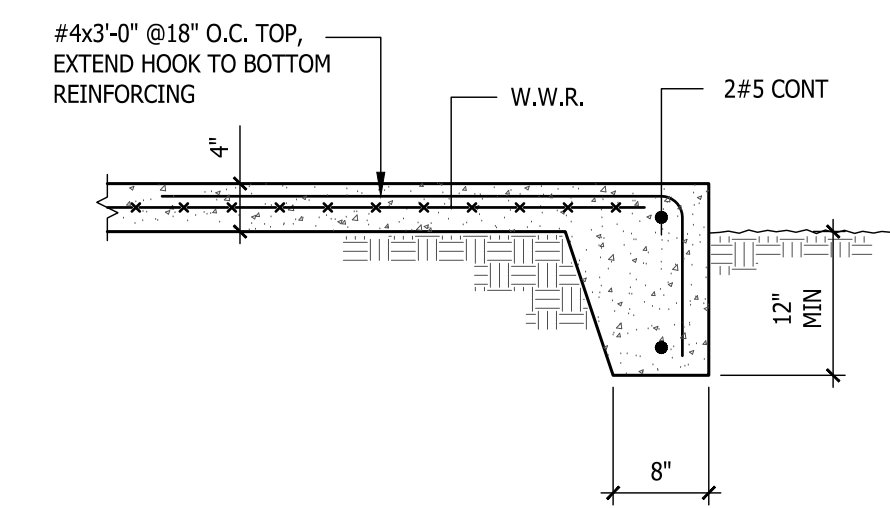
6 TYPICAL MASONRY SILL DETAIL



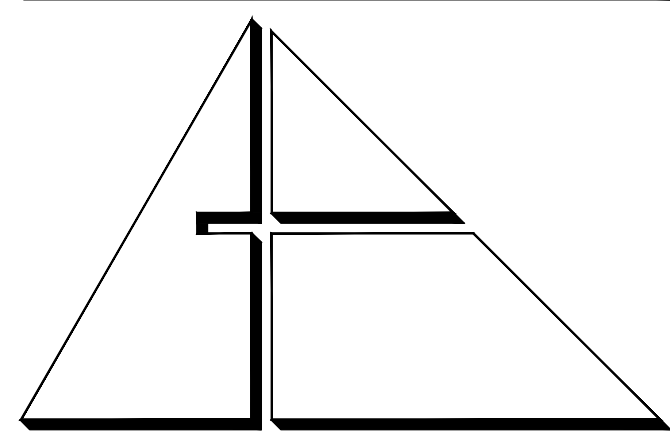
7 MASONRY CONTROL JOINT DETAILS



8 TYPICAL HORIZONTAL REINFORCING CONCRETE TIE BEAMS, WALLS, AND FOOTINGS



9 SLAB EDGE DETAIL AT PERIMETER



ALLEGUEZ ARCHITECTURE, INC.
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PROJECT: BNI # 12M18.02

CITY OF MIAMI
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14950 NE 11TH AVENUE

MIAMI, FL

DRAWING TITLE:
DETAILS

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George N. Khoury, P.E. Fla. Reg. No. 55479
BNI Project No.: 12M18.02

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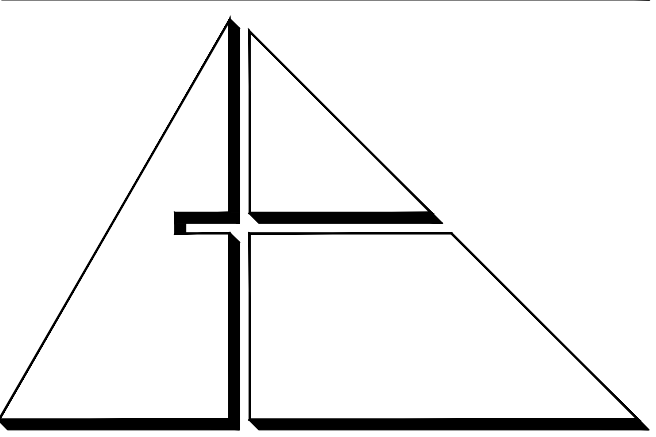
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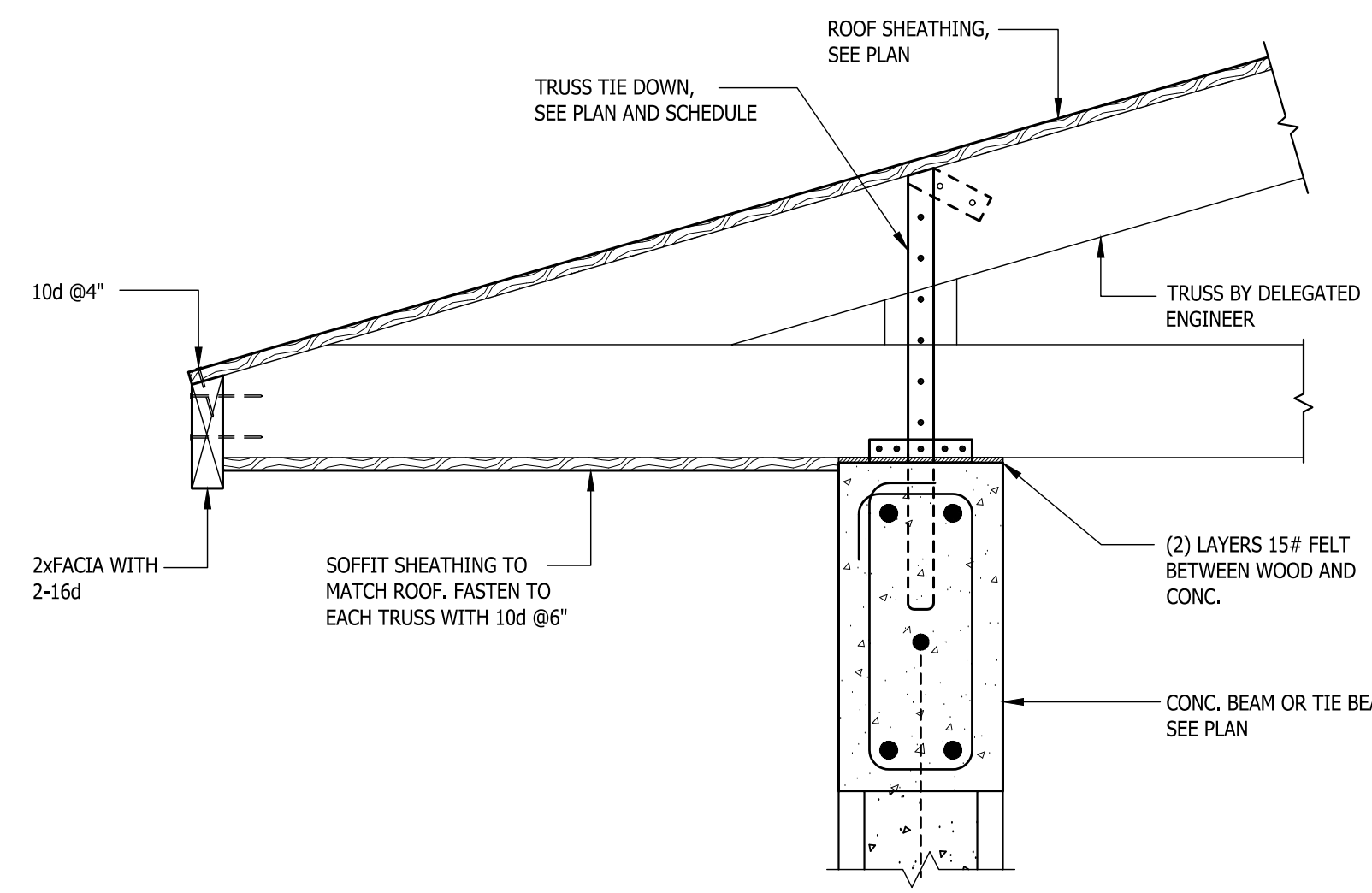
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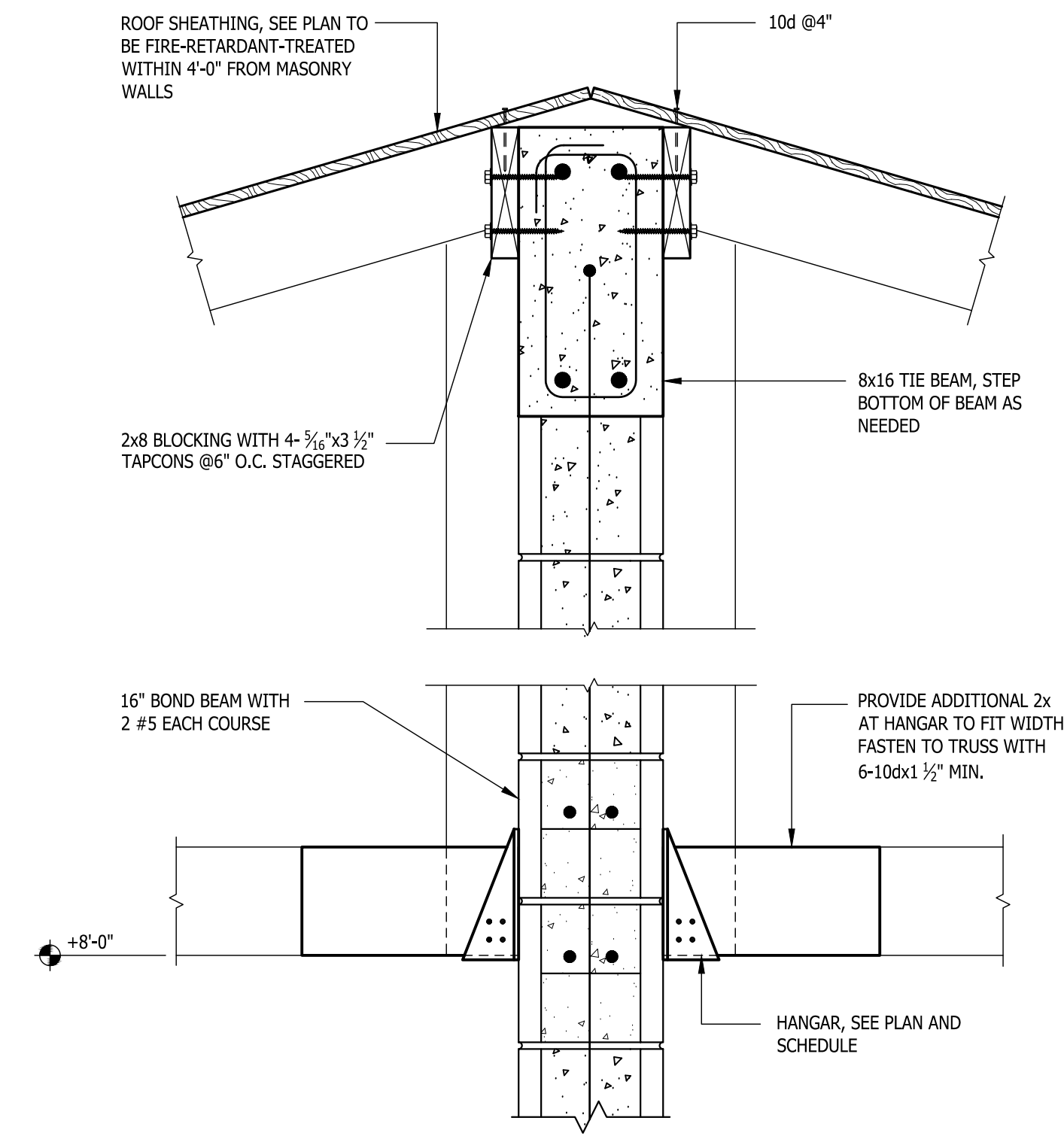


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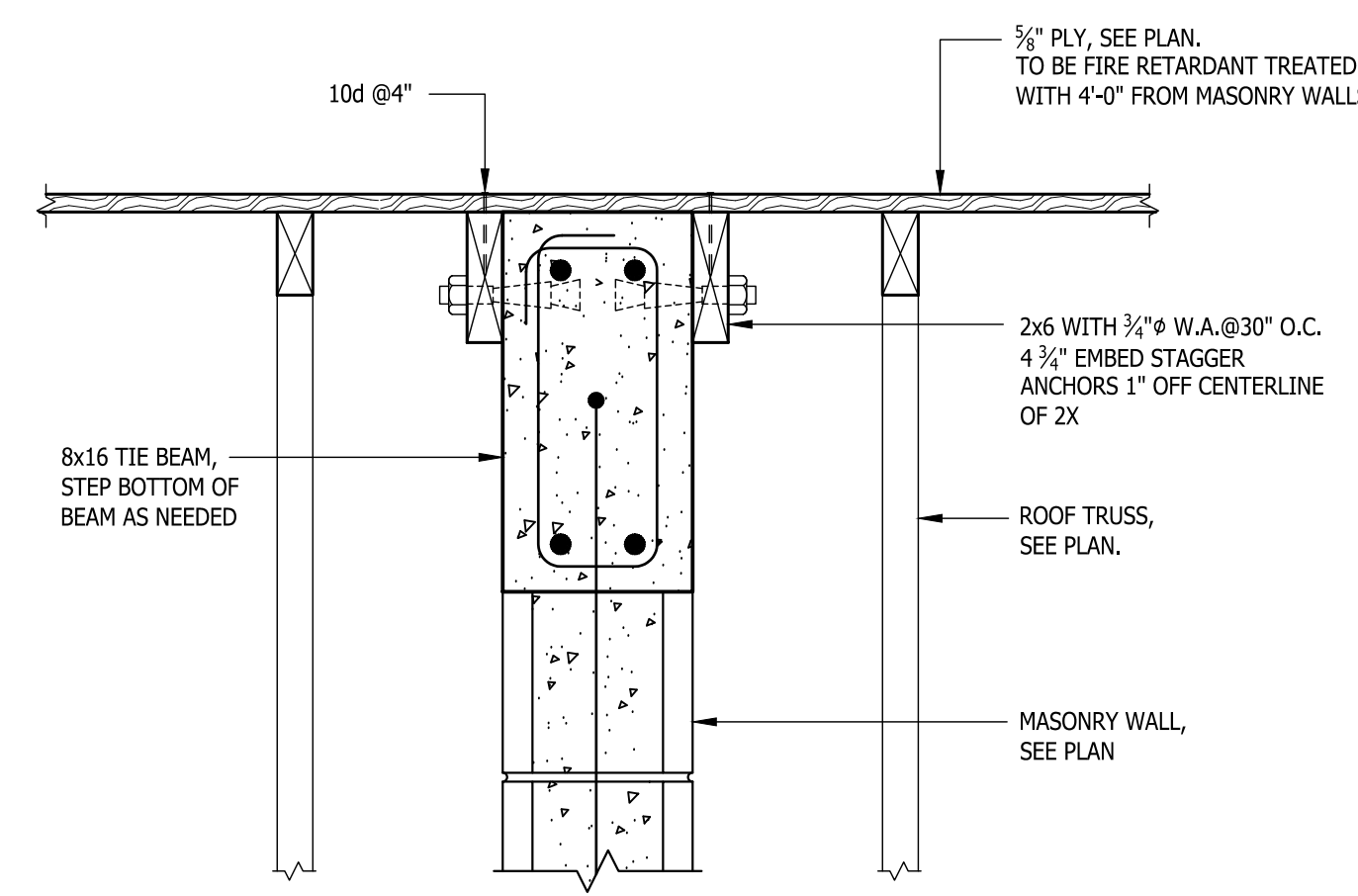
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1 ROOF TRUSS AT PERIMETER WALL
 SCALE: 1 1/2"=1'-0"

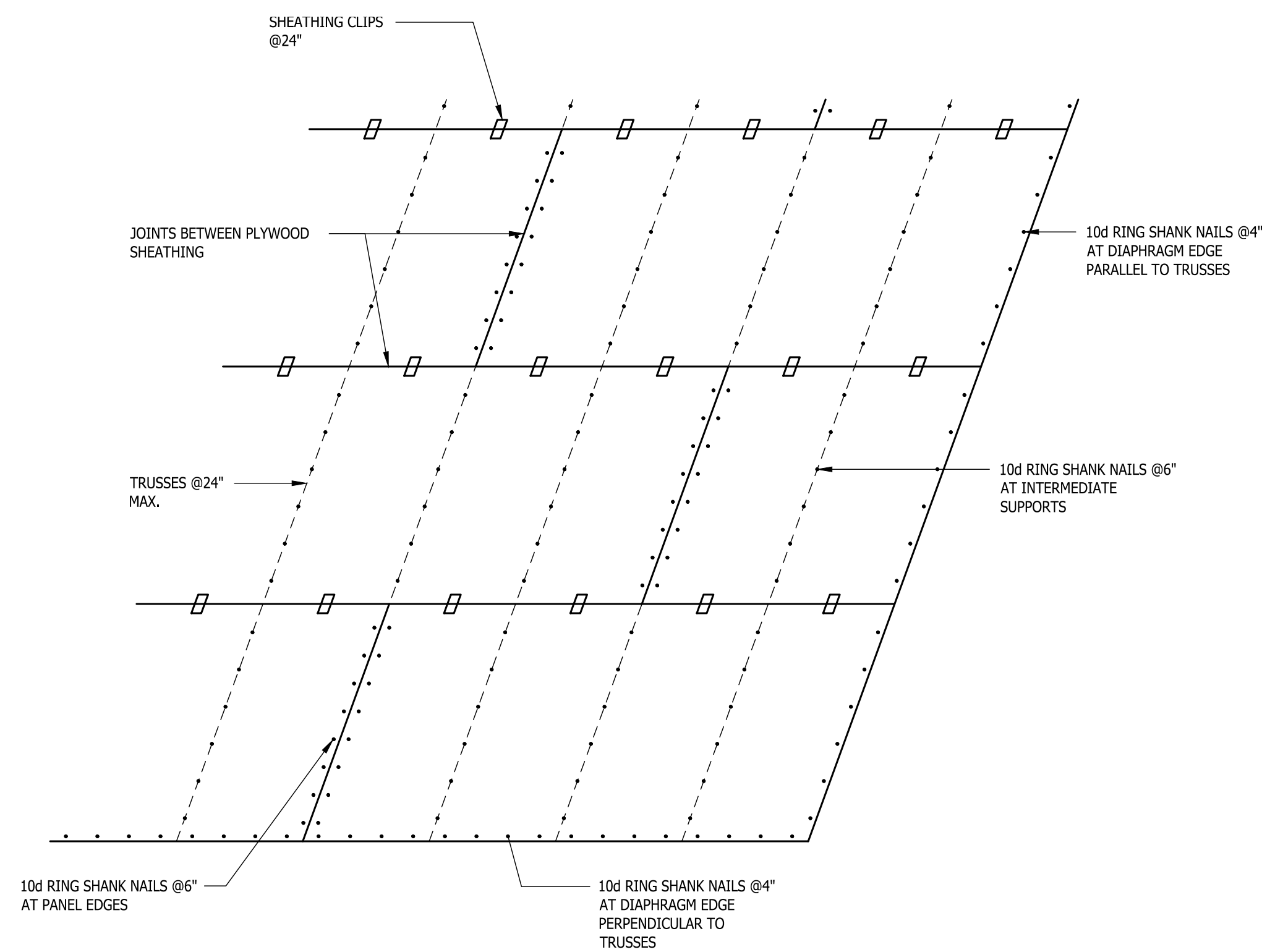


2 INTERIOR MASONRY WALL PERPENDICULAR TO TRUSS
 SCALE: 1 1/2"=1'-0"

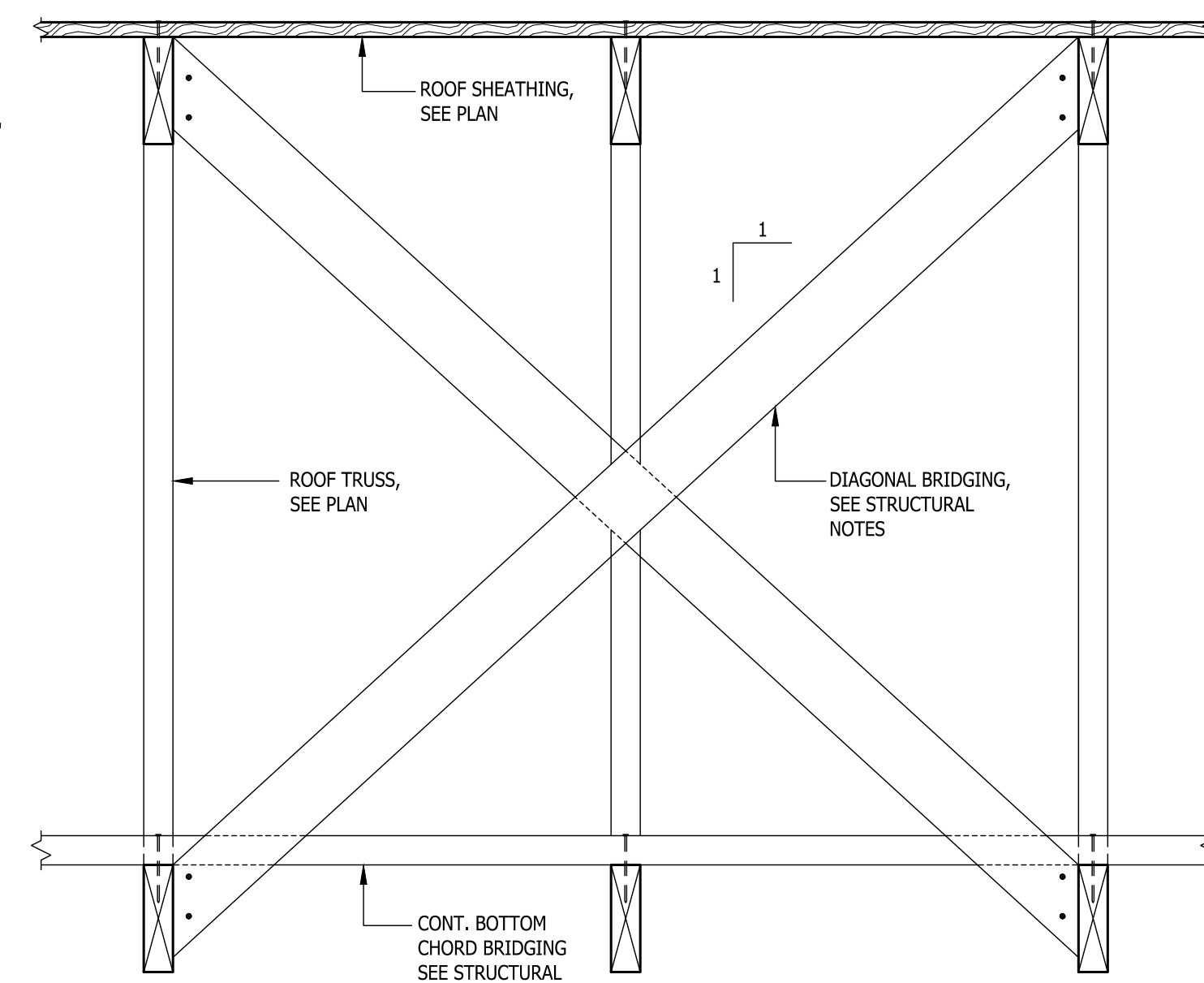


3 INTERIOR MASONRY WALL WALL PARALLEL TO TRUSS
 SCALE: 1 1/2"=1'-0"

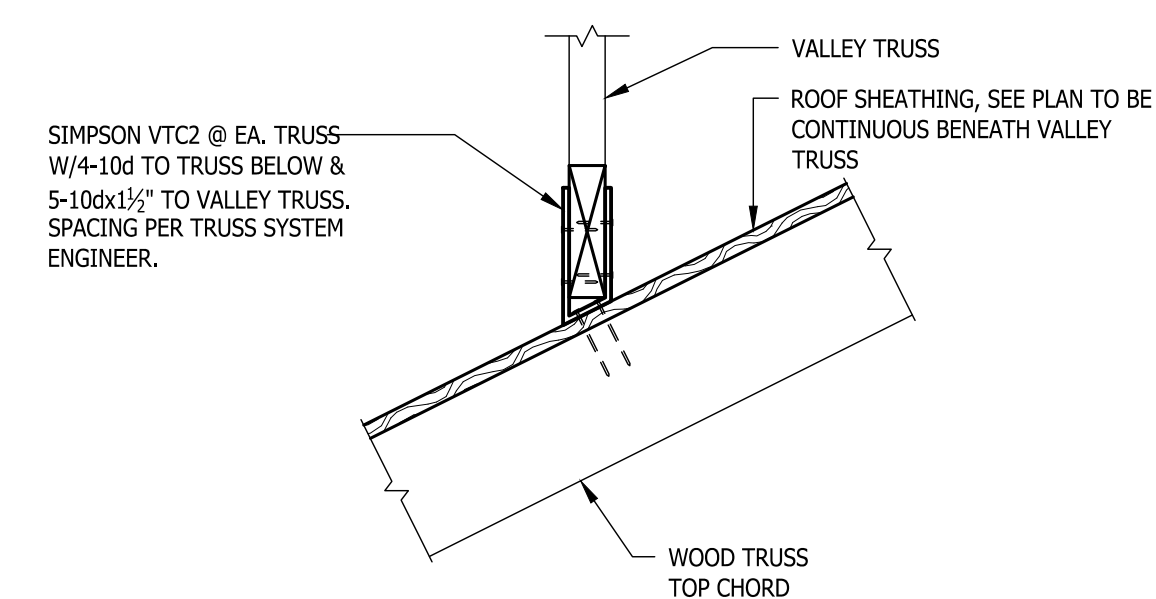
4 NOT USED



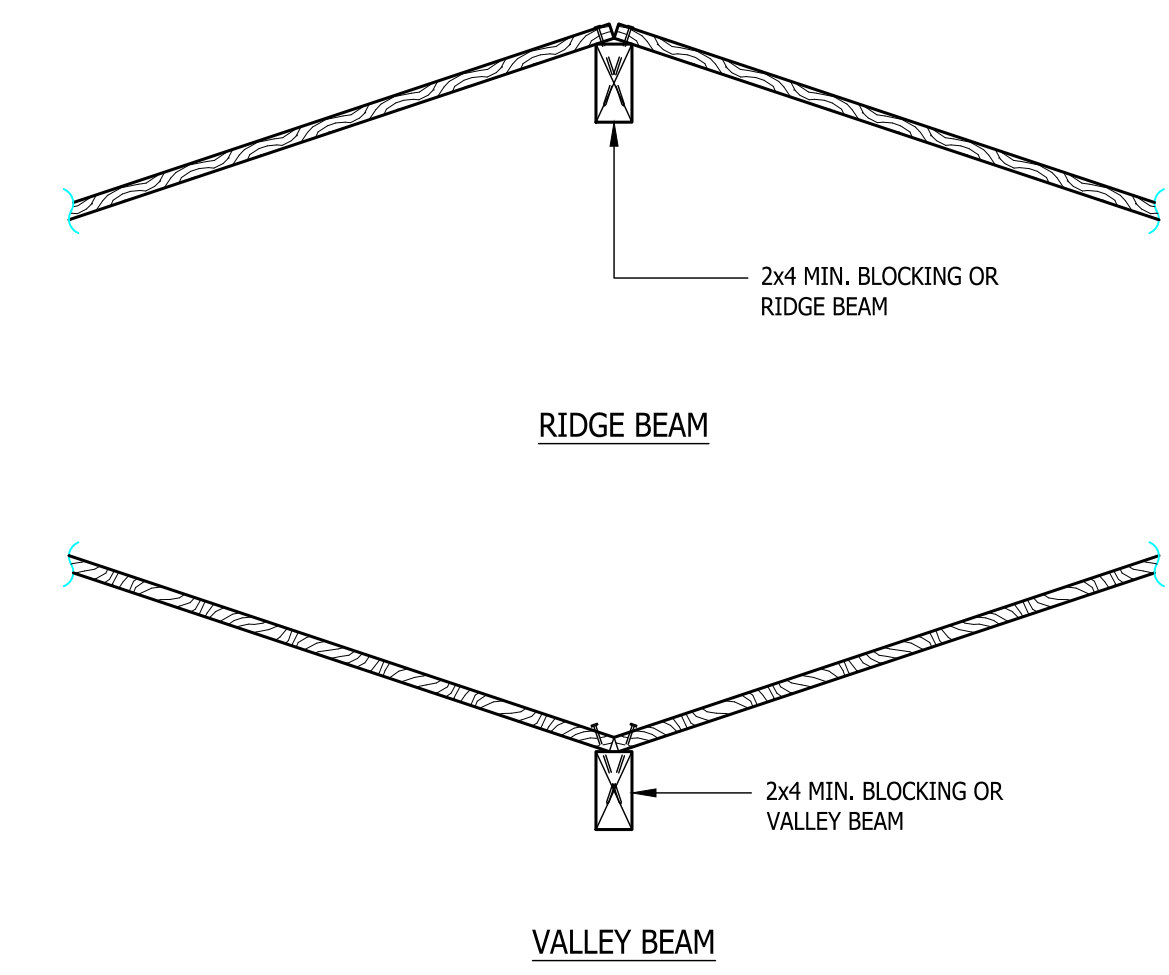
5 TYPICAL ROOF NAIL PATTERN BLOCKED DIAPHRAGM
 SCALE: 1"=1'-0"



6 TYPICAL TRUSS BRIDGING
 SCALE: 1 1/2"=1'-0"



7 VALLEY TRUSS CONNECTION
 SCALE: 1 1/2"=1'-0"



8 TYPICAL BEAM DETAIL
 SCALE: 1 1/2"=1'-0"

SIMPSON TIE DOWN CONNECTION SCHEDULE							
MARK	TYPE	UPLIFT CAPACITY	SHEAR CAPACITY	FASTEN TO MEMBER	FASTEN TO SUPPORT	FL#	NOTES
A	HETAL 20	1,810#	415#	14-10d x 1 1/2"	-	11473.5	
B	HETAL 12	1,085#	415#	10-10d x 1 1/2"	-	11473.5	
C	MGT	3,965#	-	22-10d	1-3/8" #	11470.7	GIRDER TRUSS, USE ANCHOR AS RECOMMENDED BY SIMPSON TO ACHIEVE FULL UPLIFT CAPACITY.
D	HU210-2	1,810#	-	10-10d	18-3/4" x 1 3/4" TITEN	10531.18	SEE 2/5-4.0

TIE DOWN CONNECTION NOTES:
 1. CONNECTIONS BETWEEN TRUSSES BY DELEGATED ENGINEER.
 2. REFER TO TRUSS PLACEMENT PLAN, BY DELEGATED ENGINEER, FOR TRUSS BEARING LOCATIONS.
 3. INSTALL THE DOWNS PER MANUFACTURER'S INSTRUCTIONS.

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