REPORT OF A GEOTECHNICAL EXPLORATION

NW 24 AVENUE IMPROVEMENTS NW 24TH AVENUE, FROM NW 28TH STREET TO NW 30TH STREET NW 25TH AVENUE, FROM NW 31ST STREET TO NW 32ND STREET NW 32ND STREET, FROM NW 22ND AVENUE TO NW 23RD AVENUE

CITY OF MIAMI PROJECT No. B-30723 MIAMI-DADE COUNTY, FLORIDA

- Prepared for REYNOLDS SMITH AND HILLS, INC.
6161 Blue Lagoon Drive, Suite 200
Miami, Florida 33126

- Prepared by HR ENGINEERING SERVICES, INC.
7815 NW 72nd Avenue
Medley, Florida 33166

HRES Project No. HR15-1138R

July 11, 2016

Reynolds Smith and Hills, Inc. 6161 Blue Lagoon Drive, Suite 200 Miami, Florida 33126

Attention: Mr. Samuel Gonzalez, P.E.

Vice President, Miami Office Leader

Subject: Report of a Geotechnical Exploration

NW 24th Avenue Improvements

NW 24th Avenue, from NW 28th Street to NW 30th Street NW 25th Avenue, from NW 31st Street to NW 32nd Street NW 32nd Street, from NW 22nd Avenue to NW 23rd Avenue

City of Miami

Project No. B-30723

HRES Project No. HR15-1138R

Dear Sam:

HR Engineering Services, Inc. (HRES) has completed one South Florida Water Management District (SFWMD) Usual Open-Hole Constant Head Percolation test and three asphalt cores at the subject site. This report briefly outlines our field testing techniques and presents the data obtained.

FIELD TESTING - PERCOLATION TESTING

One SFWMD Usual Open - Hole Constant Head Percolation test was performed at the subject site. Percolation Test P-1 was conducted at one depth interval: from 0 to 15 feet. The reported location of the percolation test (northing and easting) is approximate. Attached is the Field Exploration Plan showing the percolation test location.

The location of the percolation test was selected by your office. The test was located in the field by our personnel using tape measurements from existing landmarks.

The percolation test was performed in general accordance with the procedures outlined in the South Florida Waste Management District Permit Information Manual (Volume IV), Usual Open-Hole Test as follow:

• The percolation test was performed in 6.0-inch diameter boreholes, drilled to a 15 feet depth.

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• To perform the percolation test, a PVC slotted 6-inch diameter casing was placed in the hole and then water was pumped out as an attempt to clean the borehole before testing. Prior starting the test, water was pumped into the hole as an attempt to raise the water level in the borehole to the ground surface. Once the inflow stabilized with the outflow rate, the average pumping rate and the maximum level of the water obtained in the borehole (with this stabilized flow) was recorded.

The hydraulic conductivity value was then calculated and reported in units of cubic feet per second, per square foot, per foot of head (cfs/ft²-ft of head). The calculated hydraulic conductivity value was:

• From 0 to 15 feet: 4.5E-05 cfs/ft2-ft of head.

This conductivity value is an ultimate value. An appropriate factor of safety should be employed in any storm water or other subsurface drainage design computations. The percolation test result is attached to this report.

FIELD TESTING - PAVEMENT CORES

Three pavement cores were performed to a maximum depth of 2.8 inches. The asphalt cores were obtained for thickness measurement. In addition, a total of three SPT borings (one per pavement core location) were performed as an attempt to measure the thickness of the base and stabilized subgrade down to a depth of 2 feet. Also, the relative densities of both base and stabilized subgrade were obtained.

After performing each asphalt coring, a truck mounted drilled rig was placed to perform SPT testing at the coring location. The testing was conducted to a depth of 2 feet, measured from the bottom of the asphaltic layer.

Samples of the base and stabilized materials were obtained in addition to the blows/foot ("N") counts during the penetration. The blows/foot values are used to estimate the approximate in-situ density of the materials encountered. The following is a summary of our visual description of the materials found and their in-situ densities based on correlations with SPT values.

<u>Base</u> –The base layer generally consisted of very dense to medium dense light brown silty fine sand with some limerock with an assigned AASHTO classification of A-1-b (from visual classification). The base material presented "N" values ranging from 5/6" to 30/6" blows/foot. The thickness of the base was measured from the slit-spoon sampler. The base thickness is approximately 6.0 inches.

<u>Stabilized Subgrade</u> – The stabilized subgrade layer at C-1 and C-2, consisted of medium dense brown silty fine sand with some limerock with an assigned AASHTO classification of A-1-b (from visual classification). The stabilized subgrade layer at C-3, consisted of medium dense brown silty fine sand with traces of limerock with an assigned AASHTO classification of A-2-4 (from visual classification). The minimum thickness of the layer is approximately 12.0 inches.

The pavement evaluation condition data table and photos of the locations and core samples are provided in Appendix B.

The following table summarizes the thickness measurements:

Summary of Pavement Thickness Measurements

Core No.	Total Asphalt Thickness, inches	Base Thickness, inches	Stabilized Subgrade Thickness, inches	
C-1	1.7	6.0 <i>(A)</i>	>12.0 <i>(A)</i>	
C-2	1.0	6.0 <i>(A)</i>	>12.0 <i>(A)</i>	
C-3	2.8	6.0 <i>(A)</i>	>12.0 <i>(B)</i>	

Notes:

- (A): The material encountered consist of silty fine sand with some limerock and is classified as A-1-b material.
- (B): The material encountered consist of silty fine sand with traces of limerock and is classified as A-2-4 material.

We have enjoyed assisting you on this project and look forward to serving as your geotechnical consultant on the remainder of this project and on future projects. If you have any questions concerning this report, please call our office at (305) 888-8880.

Principal Geotechnica Florida Registration 4

Sincerely,

HR ENGINEERING SERVICES, INC.

Rodrigo A. Alba, E.I. Project Manager

Distribution:

Addressee (1) File (1)

APPENDIX A:

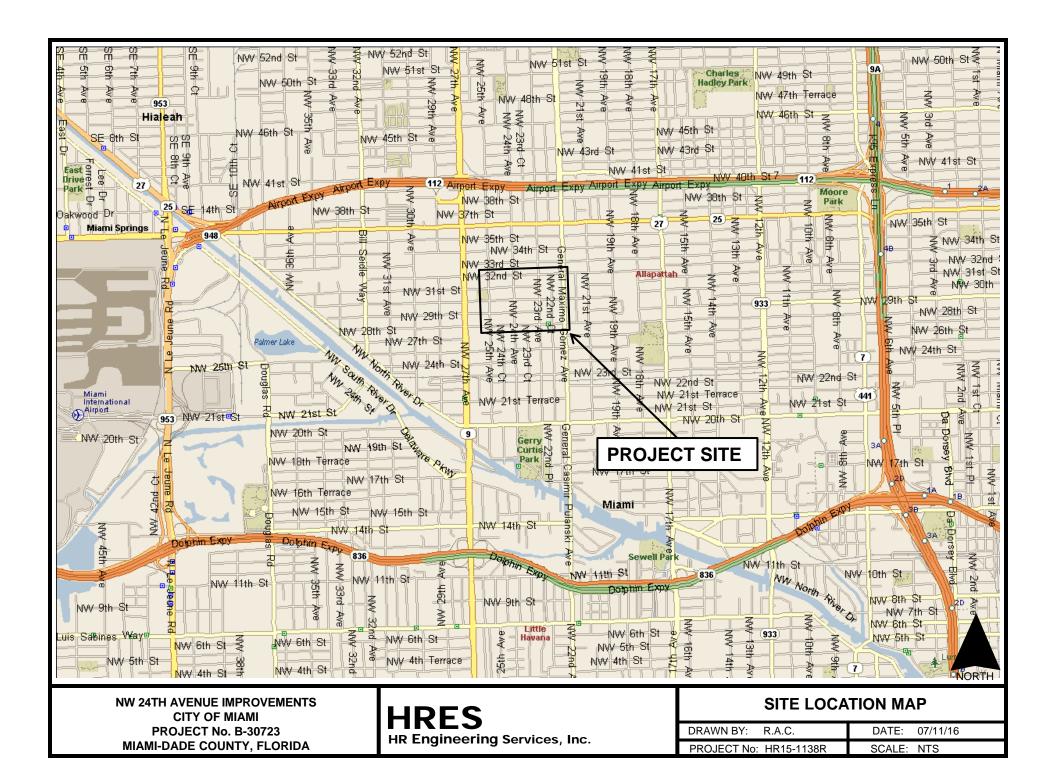
Site Location Map
Field Exploration Plans
Summary of Percolation Test and Asphalt Core Locations
Summary of Percolation Test Results

APPENDIX B:

Pavement Evaluation and Condition Data Table Photos of Asphalt Core Location and Sample

APPENDIX A

SITE LOCATION MAP
FIELD EXPLORATION PLANS
SUMMARY OF PERCOLATION TEST AND ASPHALT CORE LOCATIONS
SUMMARY OF PERCOLATION TEST RESULTS





NW 24TH AVENUE IMPROVEMENTS
CITY OF MIAMI
PROJECT No. B-30723
MIAMI-DADE COUNTY, FLORIDA

HRES
HR Engineering Services, Inc.

FIELD EXPLORATION PLANS

DRAWN BY: R.A.C.	DATE: 07/11/16
PROJECT No: HR15-1138R	SCALE: NTS



NW 24TH AVENUE IMPROVEMENTS
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FIELD EXPLORATION PLANS

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SUMMARY OF PERCOLATION TEST AND ASPHALT CORE LOCATIONS NW 24TH AVENUE IMPROVEMENTS

NW 24TH AVENUE, FROM NW 28TH STREET TO NW 30TH STREET NW 25TH AVENUE, FROM NW 31ST STREET TO NW 32ND STREET NW 32ND STREET, FROM NW 22ND AVENUE TO NW 23RD AVENUE CITY OF MIAMI

PROJECT No. B-30723

MIAMI-DADE COUNTY - FLORIDA HR ENGINEERING SERVICES, INC. HRES PROJECT No. HR15-1138R JULY 11, 2016

TEST	PLANE CO	ORDINATES	STATION	OFFSET	REFERENCE	
NUMBER	NORTHING	EASTING		ft.	BASELINE	
C-1	534649.658	907564.814	NA	NA	NA	
C-2	535546.972	906855.503	NA	NA	NA	
P-1	535778.031	908463.064	NA	NA	NA	
C-3	535778.031	908467.999	NA	NA	NA	

Notes:

Plane coordinates were taken using a hand-held GPS and are approximate within 10 feet.

NA: Not Available

SUMMARY OF PERCOLATION TEST RESULTS USUAL OPEN-HOLE - SOUTH FLORIDA WATER MANAGEMENT DISTRICT METHOD NW 24TH AVENUE IMPROVEMENTS

NW 24TH AVENUE, FROM NW 28TH STREET TO NW 30TH STREET NW 25TH AVENUE, FROM NW 31ST STREET TO NW 32ND STREET NW 32ND STREET, FROM NW 22ND AVENUE TO NW 23RD AVENUE CITY OF MIAMI

PROJECT No. B-30723
MIAMI-DADE COUNTY, FLORIDA
HR ENGINEERING SERVICES, INC.
HRES PROJECT NO. HR15-1138R
JULY 11, 2016

TEST No.	TEST DATE	NORTHING	EASTING	DEPTH TO WATER BEFORE TEST, H	DEPTH TO WATER DURING TEST,	HEAD, Du ft.	HOLE DEPTH ft.	HOLE DIAMETER, d		E OF W, P	k, HYDRAULIC CONDUCTIVITY cfs/ft^2-ft. Head
				ft.	ft.			inches	gpm	cfs	
P-1	06/10/16	535778.031	908463.064	6.0	0.0	6.0	15.0	6.0	2.3	0.00512	4.5E-05

for 0 to 15 ft., $K_{15} = P/3.1416 * d * Du \{ Du/2 + Ds \}$, where Ds = Hole Depth - H Plane coordinates were taken using a hand-held GPS and are approximate within 10 feet.

APPENDIX B

PAVEMENT EVALUATION AND CONDITION DATA TABLE PHOTOS OF ASPHALT CORE LOCATION AND SAMPLE

PAVEMENT EVALUATION AND CONDITION DATA CITY OF MIAMI

NW 24TH AVENUE BETWEEN NW 28TH STREET AND NW 30TH STREET, NW 25TH AVENUE BETWEEN NW 31ST STREET AND NW 32ND STREET AND NW 32ND STREET BETWEEN NW 22ND AVENUE AND NW 23RD AVENUE PROJECT No. B-30723

NW 24TH AVENUE IMPROVEMENTS MIAMI-DADE COUNTY, FLORIDA HR ENGINEERING SERVICES, INC. HRES PROJECT No. HR15-1138R

Typical Section No. Eloydis Cruz Date: July 10, 2016 Cored By: Page: FPID No.: Name: NW 24th Avenue Improvements Lanes: Shoulder Type & Cond: County: Miami-Dade County S.R. No. Paved From Inside: N/A To. Median Curbed? Beg. Sta. Length: Outside: End Sta: Curb & Gutter? Other? Crack HRES Pavement Layer (inches) Rut Core Sub Base Rut Depth Wheel Base Pavement Core Northing Easting Lane Lane Description Top Depth Location Cross Slope (ft/6ft) No. Path (inches) (inches) Type Class Extent Condition No. FC-2 FC-4 S-I Binder Type II ABC-1 Core Length (in) (ft) (ft)

1.7

1.0

2.8

6.0 (A)

6.0 (A)

6.0 (A)

>12.0 (A)

>12.0 (A)

>12.0 (B)

Ρ

Р

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C-3

C-1

C-2

A = The material encountered consisted of silty fine sand with some limerock and was visually classified as A-1-b material

B = The material encountered consisted of silty fine sand with traces of limerock and was visually classified as A-2-4 material

NW 24th Ave. NB lane at 4.0 ft. from EOP

NW 25th Ave. NB lane at 3.5 ft. from EOP

NW 32nd St. WB lane at 4.0 ft. from EOP

Υ

P = Poor condition

3

EOP = Edge of Pavement

534649.7 907564.8

535547.0 906855.5

535778.0 908468.0

NB

NB

WB





Northing: 534649.7 Offset: 907564.8

Core No. C-1 Core Thickness: 1.7 in.

PROJECT:HR15-1138R SHEET No.

NW 24TH AVENUE IMPROVEMENTS
NW 24TH AVENUE, FROM
NW 28TH STREET TO NW 30TH STREET
PROJECT No. B-30723
MIAMI-DADE COUNTY, FLORIDA

HR ENGINEERING SERVICES, INC. 7815 NW 72nd AVENUE MEDLEY, FLORIDA 33166 PHOTOS OF ASPHALT CORE LOCATION AND SAMPLE

DRAWN BY: HRR DATE: 07/11/16





Northing: 535547.0 Offset: 906855.5

Core No. C-2 Core Thickness: 1.0 in.

NW 24TH AVENUE IMPROVEMENTS NW 25TH AVENUE, FROM NW 31ST STREET TO NW 32ND STREET PROJECT No. B-30723 MIAMI-DADE COUNTY, FLORIDA

HR ENGINEERING SERVICES, INC. 7815 NW 72nd AVENUE MEDLEY, FLORIDA 33166

PHOTOS OF ASPHALT CORE LOCATION AND SAMPLE				
DRAWN BY: HRR	DATE: 07/11/16			
PROJECT:HR15-1138R	SHEET No.			





Northing: 535778.0 Offset: 908468.0

Core No. C-3 Core Thickness: 2.8 in.

NW 24TH AVENUE IMPROVEMENTS NW 32ND STREET, FROM NW 22ND AVENUE TO NW 23RD AVENUE PROJECT No. B-30723 MIAMI-DADE COUNTY, FLORIDA

HR ENGINEERING SERVICES, INC. 7815 NW 72nd AVENUE MEDLEY, FLORIDA 33166

PHOTOS OF ASPHALT CORE LOCATION AND SAMPLE

DRAWN BY: HRR

DATE: 07/11/16

PROJECT:HR15-1138R SHEET No.