ATTACHMENT "SECTION 03300"

CAST-IN-PLACE CONCRETE, REINFORCING AND FORMWORK

(17 Pages)

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PART 1 GENERAL

1.01 DESCRIPTION

- A. Work included: Provide all labor, materials, equipment, fabrication, incidentals, transportation, placing and supervision necessary to complete all cast-in-place concrete work, its finishing, and all related work called for by the Plans and/or Specifications, or reasonably inferable from either or both, as needed for a complete and proper installation.
- B. Related work: Work affecting this Section includes, but is not limited to:
 - 1. Shop Drawings Per General Conditions and as specified herein.
 - 2. Materials and storage thereof.
 - 3. Reinforcing-Bar and fabric.
 - 4. Accessories of every nature, including form tie system.
 - 5. Formwork and removal thereof, including shoring and reshoring.
 - 6. Concrete proportions and mixes.
 - 7. Placing of concrete.
 - 8. Admixtures.
 - 9. Joints, metal joint screeds and joint fillers.
 - 10. Finishes of all types.
 - 11. Protection and curing.
 - 12. Patching.
 - 13. Laboratory Testing.

1.02 QUALITY ASSURANCE

- A. Unless otherwise indicated, all materials, workmanship and practices shall conform to the requirements of ACI 301-96 "Specifications for Structural Concrete for Buildings", except as modified by supplemental requirements hereinafter.
- 1.03 STANDARDS
 - A. ACI 301-10 Specifications for Structural Concrete.
 - B. ACI 318-11 Building Code Requirements for Reinforced Concrete.
 - C. Florida Building Code, latest edition.
 - D. ACI 117-10 Standard Specifications for Tolerances for Concrete Construction and Materials.

PART 2 PRODUCTS

2.01 MATERIALS

- A. Materials for Concrete:
 - 1. Cement shall conform to the following: Portland Cement ASTM C150, normal, type I or type II. Provide domestic cement of one type and from same source for entire project.
 - 2. Mineral Admixtures:
 - a. Fly Ash: Shall conform to ASTM C618. 20% maximum of total cementitious weight.
 - b. Ground Blast Furnace Slag: Shall conform to ASTM C989-93.
 30% maximum of total cementitious weight.
 - 3. Chemical Admixtures: The following admixtures are permitted, but require written approval from the Engineer:
 - a. Air Entraining Admixture: Comply with ASTM C260. "Specifications for Air-Entraining Admixtures for Concrete."
 - b. Water Reducing Admixture: Comply with ASTM C494 "Specifications for Chemical Admixtures for Concrete", Type A and compatible with air entraining admixture.
 - c. Water Reducing and Retarding Admixture: Comply with ASTM C494, "Specifications for Chemical Admixtures for Concrete", Type D, and compatible with air entraining admixture.
 - d. High Range Water Reducing Admixture: Comply with ASTM C494, "Specifications for Chemical Admixtures for Concrete", Type F or G, and compatible with air entraining admixture. (Including superplasticizer to reduce water content.)
 - e. Admixtures containing added calcium chloride are not permitted.
 - 4. Aggregates: Shall conform to ASTM C33 and shall be quarried/mined in fresh water. Aggregates from salt water or brackish water are not permitted.
 - a. Fine aggregate shall be silica (quartz) sand. Manufactured sand and screenings are not permitted.
 - b. Coarse aggregate size shall not exceed:

Concre	<u>Size</u>		
1)	Walls	3/4"	67#
2)	Beams or structural slabs not on ground	3/4"	67#
3)	Columns and all other concrete	1"	57#
4)	Drilling concrete pad or slabs on ground	1"	57#

- 5. In sanitary sewage applications, where called for in the Plans and/or specifications an antimicrobial admixture as specified below shall be utilized:
 - a. An antimicrobial agent, Con^{MIC}Shield[®], or approved equal, shall be used to render the concrete uninhabitable for bacteria growth.
 - b. Contractor shall mix the liquid antimicrobial additive with the

total water content of the concrete mix design in a proportion of 1 gallon per cubic yard. In the case of repairs to damaged concrete a proportion of 2 gallons per cubic yard shall be utilized.

- c. In some instances all of the concrete in the structure in will receive the additive and in other instances only a portion of the concrete will receive the additive. Hence, the Contractor shall apply the additive only as directed in the specific instance.
- d. Contractor shall submit a letter of certification to the Department, stating that the correct amount and correct mixing procedure was followed for all antimicrobial concrete.
- e. Con^{MTC}Shield[®] antimicrobial additive shall be as manufactured by Con^{MIC}Shield[®] Technologies, Inc.; 541 Tenth Street NW #233, Atlanta, GA 30318; Phone: 877-543-2094.
- B. Portland cement and reinforcing steel: Comply with ACI 301-10 and, with all modifications and supplements thereto listed in Part 3 of these Specifications.
- C. Burlap mats: Conform to AASHTO Specification M182. (Burleen non-staining mats.)
- D. Epoxy bonding agent: A two (2) component, solvent free, moisture insensitive structural epoxy adhesive conforming to ASTM C881-13 Type II, Sikadur 32 Hi-Mod, as manufactured by Sika Corp., Concresive 1090 Liquid by Master Builders or approved equal.
- E. Anchor bolts, nuts and washers: Conform to ASTM A449-10, hot-dip galvanized.
- F. Dovetail slots: Galvanized steel, 22 gauge, 1 x 1 inch, with 5/8-inch throat, fiber filled.
- G. Forms:
 - 1. Plywood Forms: PS-1, B-B Concrete Form, Class I, exterior type, mill oiled and edge sealed. Thickness shall be as required to support concrete at the rate placed, but not less than 3/4-inch.
 - 2. Steel Forms: Uncoated steel, 3/16-inch minimum thickness, fabricated to close tolerances, protected only by the specified release agent, braced so as not to dent, bend or dimple under wet concrete loads, vibrator impact and tool impact. Maintain steel forms in rust free condition by use of steel wool and light grinding, followed by coats of the specified release agent. Forms should be adjustable to be brought into true alignment without steps or ridges.
- H. Form release agent:
 - 1. For plywood forms use a natural non-petroleum base, non-staining and non-retarding release agent that will effectively prevent absorption of moisture and prevent bond with concrete, and leaves the concrete with a

paintable surface.

- 2. For steel forms, use an approved material that will not stain, color or otherwise affect the finish of the concrete. Form coating shall not be detectable on finished surfaces.
- 3. Round column forms: Provide seamless fiber forms with the three plies nearest to the interior surface of the form deckled or scarfed and overlapped to minimize spiral gaps or seams on the column surface.
- I. Form Ties: Steel rod type with integral waterstops and cones, and with ends or end fasteners that can be removed without spalling the concrete and which leave a hole equal in depth to the required reinforcement clearance, but not less than 2 inches from the formed face of the concrete. Wire tie, banding wire and wood spreaders will not be permitted.
- J. Form Inserts:
 - 1. Bevel or chamfer strips: Wood or non-staining plastic, 3/4-inch wide on each leg at exposed edges of concrete members, unless otherwise noted on plans.
 - 2. Tongue and Groove Joint Forms: Minimum 24 gauge with steel stakes and splice plates. Forms shall be designed for joints not to receive a poured seal.
 - 3. Pipe hangers and other utility supports: AISI Type 316 stainless steel.
- K. Non-Shrink Grout: Non-shrink, non-metallic grout conforming to ASTM C1107-13 Grade B or Grade C only. Grout must meet ASTM C1107-13 at a temperature range of 50°F to 90°F at a flowable consistency.
- L. Grout for Surface Repair and Bond Coat:
 - 1. For repair, one part Portland cement to two parts fine sand, and a 50% of water and 50% Acryl 60 or equal (Thoroseal or Acryl Set Bonding Agent by Master Builders) to produce a stiff mortar.
 - 2. For bond coat, one part Portland cement to one part sand, and a 50% of water and 50% Acryl 60 or equal (Thoroseal or Acryl Set Bonding Agent) to produce a slurry mix.
- M. Moisture Barrier: Kraft paper and glass reinforcing fibers sandwiched between 2 layers of polyethylene film with a permeance rating of maximum 0.1 as per ASTM E96-00, Procedure A.
- N. Preformed Expansion Joint Filler: Non-extruding type, self expanding cork, 3/4-inch, 1-inch, and 1-1/2-inch cork (not to be used for sidewalks), conforming to plans or as otherwise noted on drawings, conforming to the requirements of ASTM D1752-04a (2013), Type II, and compatible with joint sealant compound.
- O. Joint Sealant Compound: Non-sag, 2 component, solvent free, moisture insensitive, flexible, epoxy resin conforming to the requirements ASTM C920-14 Type M, Grade NS. Additionally, the sealant must be recommended by the manufacturer to perform under continuous immersion in water.
- P. Polyurethane Elastomeric Sealant: Sikaflex-2c, NS/SL or approved equal. Provide a 2-component, premium-grade, polyurethane-based, elastomeric

sealant. It is principally a chemical cure in a non-sag and self-leveling consistency. Sealant shall meet ASTM C920-14 and Federal Specifications TT-S-00227E.

- 1. Joint Movement: +50%.
- Q. Waterstops:
 - 1. Volclay Waterstop-RX or approved equal. Flexible strip of bentonite waterproofing compound in coiled form.
 - a. Chemical Composition:
 - 1) Butyl Rubber-Hydrocarbon: 24.9% by weight; ASTM D297.
 - 2) Bentonite: 75% by weight; SS-S-210-A.
 - 3) Volatile Matter: Below 1%; ASTM D6.
 - Waterstop shall not contain any asbestos fibers or asphaltics.
 - b. Physical Properties:
 - 1) Specific Gravity: 1.57; ASTM D71.
 - 2) Application Temperature Range: 5-125°F.
 - 3) Flash Point: 365; ASTM D93-97.
 - 4) Accelerated Aging: Maintained 99% solids.
 - 5) Dimensions: 1" x 3/4" x 16'-6"
 - 2. Polyvinyl chloride (PVC): Conforming to the requirements of U.S. Army Corps of Engineers Specification CRD-C-572 and of the following type:
 - a. Expansion Joints: 9-inches by 3/8-inch, ribbed center bulb.
 - b. Construction Joint: 9-inches by 3/8-inch, flat ribbed.
 - c. Only where specified on Plans at construction and expansion joints: 9-inches by 3/8-inch, split ribbed.
 - d. Install waterstops as shown as manufactured structures.
- R. Fiber Reinforcement: Fiber reinforcement shall not be used in the concrete unless ordered by the Engineer in writing. It shall consist of 100% virgin polypropylene fibrillated fiber-dosage of 2 lbs. per cubic foot.
 - 1. Compressive Strength: 1 psi (.006895 M Pa), ASTM C39.
 - 2. Flexural Strength: 288 psi (2.0 M Pa) after 7 days, 390 psi (2.7 M Pa) after 28 days; ASTM C78.
 - 3. Splitting Tensile Strength: 194 psi (1.3 M Pa) after 7 days, and 290 psi (2.0 M Pa) after 28 days; ASTM C496.
 - 4. Source: Fibermesh Micro-Reinforcement System by Fibermesh Company, Division of Synthetic Industries, Inc., or approved equal.
- S. Provide other materials, not specifically described but required for a complete and proper installation, as selected by the Contractor subject to the approval of the Engineer.
- T. A shrinkage reducing admixture (Teraguard) or equivalent at the rate of 2.2% by weight of cement may be used in the concrete to meet the shrinkage limitations.
- U. To protect the concrete slab against the elements, the Engineer may direct the Contractor to spray an evaporation retarder on the finished concrete slab

immediately behind the cement finishing process at no additional cost to the Department. This is not a curing compound.

PART 3 EXECUTION

3.01 SURFACE CONDITIONS

A. Examine the areas and conditions under which work of this Section will be performed. Correct conditions detrimental to timely and proper completion of the Work.

3.02 SUPPLEMENTAL REQUIREMENTS

- A. All phases of concrete construction, including materials formwork, and all other related procedures shall comply with the most stringent allowed tolerances of ACI-301 and ACI-117 Standards (Latest Edition) Non compliance with these standards will cause full rejection of any work done.
- B. Comply with ACI 301-10 and with all modifications and supplements thereto listed herein. In addition to the ACI Standards on finished concrete, the Engineer will only approve quality finished concrete which in his opinion is ready to receive a grout finish, paint or liquid membrane.
- C. The following modifications and supplements to ACI 301-10 shall also apply to the work.
 - General

1.

- a. These specifications cover cast-in-place structural concrete for use in buildings and appurtenances, including foundations, curbs, sidewalks, concrete pavements and utility structures, water containment tanks, and piles.
- b. Keep minimum two (2) copies of ACI 301-10 "Specifications for Structural Concrete" in field office at all times.
- 2. Proportioning and Design of Mixes:
 - a. General: Proportion concrete to meet properties as specified. Prepare mix designs for each type and strength of concrete. Submit with mix design the chemical admixture manufacturer's statement that the admixture proposed complies with the requirements of this specification. Where concrete of different strengths are specified for the same location, the higher strength concrete shall be used. Concrete proportions shall be established on the basis of previous field experience, or laboratory trial batches as specified in ACI 301-10 Sections 4.2.2 & 4.2.3.
 - b. Classes of Concrete:
 - Structural concrete of normal weight for portions of the structure that are required to be watertight containments or tremie concrete, the water/cementitious ratio shall not exceed 0.45 if exposure is to be to fresh water.
 - 2) If the concrete is exposed to salt or brackish water, or if exposed to injurious concentrations of sulfate-containing

solutions (1,500 ppm or more of Sulfate in water) or other chemically aggressive solutions, use Type II cement with Rheobuild 1000 admixture by Master Builders, or approved equal; water/cementitious ratio shall not exceed 0.34.

- 3) Other Concrete: (This would be slabs-on-grade, concrete thrust blocks, and miscellaneous concrete). The water cementitious ratio shall not exceed 0.50 to 0.55.
- 4) Minimum f'c @ 28 days shall be 3,000 psi.
- 5) Minimum f'c @ 28 days shall be 4,000 psi with a Water/Cement ratio of 0.45. (see 1) above)
- 6) Minimum f'c @ 28 days shall be 7,000 psi with a Water/Cement ratio of 0.34. (see 2) above)
- c. Slumps:
 - All structural concrete, pumped concrete and tremie concrete shall contain a High Range Water Reducing Admixture and be designed with a maximum water content of 270 pounds per cubic yard (32.36 gallons). The initial water slump prior to addition of the High Range Water Reducing Admixture shall be 2 inches maximum. Concrete at point of placement shall not exceed 10 inches. Concrete shall be non-segregating.
 - 2) Slabs including slabs-on-grade, and all other concrete shall have a maximum water content of 287 pounds per cubic yard (34.4 gallons) and have a 5-inch maximum slump with a water reducer, or water reducer and retarder admixture added.
- 3. Formwork
 - a. Earth cuts are not permitted for forms for vertical surfaces. Footings, grade beams and slab edges shall be formed. Provide moisture barrier under all slabs on grade. Lap 6 inches and tape punctures.
 - b. The contractor is responsible for the adequacy of forms and shoring including placing, fill and equipment on roof, and for safe practice in their use and removal. Submit formwork calculations, and shop drawings including shoring and reshoring. In addition, the calculations and shop drawings for formwork, shoring, and reshoring, if required by the Engineer or Building Department, shall be signed and sealed by a Professional Engineer registered in the State of Florida.
 - c. Design forms for the loads and lateral pressures resulting from the placement and vibration of concrete and for design considerations, wind loads, allowable stresses, and other applicable requirements of the Florida Building Code.
 - d. Provide form facing materials as required by the specified finish of the formed surface. Do not use facing material with raised grain, torn surfaces, worn edges, patches, dents or other defects. No form may be reused more than three times without the Department's approval. The maximum deflection permitted of facing materials reflected in concrete surfaces exposed to view is

1/240 of the span between structural members.

- Forms shall be free from surface defects, tight to prevent 1) leakage and braced to keep its position and shape when filled with concrete. Adjacent edges and end panels and sections shall be held together to provide accurate alignment and prevent forming ridges, fins, offsets or similar type defects in finished concrete. It shall be tight to prevent loss of water, cement or fines during placing and vibrating concrete. The bottom of the forms placed in continuous straight even footings or slabs shall be watertight to prevent loss of water, cement and fines during placement and vibration of concrete, a gasket may be required by the Engineer under the forms to provide water tightness at the Contractor expense. The Contractor shall not proceed to place forms for concrete work adjacent to or on top of previous placed concrete without the Engineer's approval, if the stripped forms reveals columns, walls or beams are out of level or plumb or there are cold joints or other objectionable work in the opinion of the Engineer. Contractor shall submit to the Engineer for approval, how he intends to correct or remove the defective work promptly at his expense. Contractor shall perform such corrections prior to proceeding to place concrete in the next Section.
- e. Provide positive means of adjustment (wedges or jacks) of shores and struts, and all settlement shall be taken up during concrete placing operation. Brace forms securely against lateral deflection. Do not anchor form bracing to poured concrete floors, or make holes in floor.
- f. Provide temporary openings in columns and wall forms to limit the free fall of concrete to five (5) feet. Place such openings at no more than eight (8) feet apart to facilitate placing and consolidation of concrete. Elephant trunks may be used to vertical heights of fifteen (15) feet for tremie and other purposes, if approved by the Engineer. Provide temporary openings at the bottom of wall and column forms and elsewhere as necessary to facilitate cleaning and observation immediately before concrete is placed. Blow formwork entirely clean of all saw dust, dirt, or other items not specifically intended to be a part of the final concrete. Any evidence of non-intended items in the forms is considered sufficient cause to stop concreting operation and/or require removal of concrete placed in such contaminated forms.
- g. Provide inserts, conduits, boxes, sleeves, anchors, ties, bolts, hangers, dowels, thimbles, nailers, grounds and other devices in coordination with other trades.
- h. Set anchor bolts and other embedded items accurately and hold securely until concrete is placed and set. Anchor bolts shall be galvanized and of size and length as indicated on the Plans. Bolts not sized shall be 3/4-inch diameter.
- i. Insert galvanized dovetail anchor slot in forms, in columns, beams

and slabs completely around in-fill masonry panels. Coordinate with Section 04220 Unit Masonry for spacing of dovetails.

- j. Install wall spools, wall flanges and wall anchors before placing concrete. Do not weld, tie or otherwise connect the wall spools to the reinforcing steel.
- k. Do not use pinch bars, wrecking bars or other metal tools against as-cast concrete to wedge forms loose; use only wooden wedges carefully and gradually. Driving shall be accomplished by light tapping.
- I. The Contractor is responsible for the removal of forms and shores. Concrete shall be cured in accordance with ACI 308r-01 (also see section 10 below). Do not remove forms or shores before the member has attained sufficient strength to support its weight and the loads imposed, nor sooner than listed below:
 - 1) Wall forms: 24 hours.
 - 2) Column forms: 24 hours.
 - 3) Beam and girder side forms only (not bottom form): 24 hours.
 - 4) Beam and Girder bottom forms: 7 days minimum unless otherwise approved by the Engineer.
 - 5) Slab forms: 14 days.
 - 6) Arch centers: 7 days.
 - 7) Pan joist forms: 4 days.
- 4. Reinforcement
 - a. Prior to fabrication, submit for review shop drawings showing all fabrication dimensions, bar lists and location for placing of the reinforcing steel and accessories, including spacing of reinforcing, splices (lap, welded, Cadweld and/or mechanically threaded), grade of reinforcing and name of manufacturer. Note all deviations from the Plans and use the same designation mark as shown on the Plans where possible.
 - b. Reinforcing bars: ASTM A615, Grade 60, deformed bars of USA manufacturer.
 - c. Welded wire fabric: ASTM A185, galvanized.
 - d. Metal bar supports: CRSI MSP-1, Chapter 3, Class 2, Type B stainless steel protected bar supports.
 - e. Coupler Splice Devices: Cadweld, tension couplers capable of developing the ultimate strength of the bar.
 - f. Reinforcing steel upon which unauthorized welding has been done, shall be removed and replaced at no additional cost to the Department.
 - g. Place reinforcing bars to the most stringent tolerances indicated in ACI 301 and ACI 117 (Latest Edition). Tolerances specified in those standards shall govern over any other reference code or standard.
 - h. All reinforcement at time concrete is placed shall be free of mud, oil or other materials that may affect or reduce the bond. Reinforcing with rust or mill scale will not be accepted without cleaning and/or brushing to remove scale and rust.
 - i. Support rebar and mesh reinforcing for slabs on grade 1-1/2

inches from top of slab on masonry blocks not less than 4 sq. in., having a compressive strength equal to or greater than the specified strength of the concrete being placed. Space blocks at no more than 4 feet apart each way for rebars, and no more than 3 feet apart for mesh reinforcement.

- j. Support reinforcing off from formwork for columns, walls and beams with stainless steel protected bar supports. Support slab reinforcing on #5 bars, or larger, spaced at no more than 48 inches on center. Space individual high chairs no more than 48 inches apart and support bars shall not exceed 24 inches past outermost chairs.
- k. Overlap welded wire fabric in such a manner that the overlap measured between outermost cross wires of each fabric sheet is not less than the spacing of the cross wires plus 2 inches or 6 inches, whichever is greater. Do not extend fabric through expansion and/or contraction joints, unless otherwise noted on the Plans.
- I. The minimum clear distance between parallel bars, both vertical and horizontally, shall not be less than the nominal diameter of the bars, or less than 1-1/2 times the maximum size of the aggregate, or 1 inch in beams, or 1-1/2 inches in columns, whichever is greater. Where reinforcement in beams is placed in two or more layers, the upper layer shall be placed directly above the bars in the bottom layer. Misplacement, misalignment or improper length of dowels shall be sufficient cause to require removal and reconstruction of affected work.
- m. Unless allowed by the Engineer, bending of reinforcing partially embedded in concrete is not permitted. When permitted, bending shall be in accordance with CRSI Manual of Standard Practice.
- 5. Joints and Embedded Items.
 - a. Provide premolded expansion joint filler strips of proper width and length as specified in the Plans. Place 1/2-inch expansion joint fillers every 20 feet in straight runs of walkways or sidewalks, at right angle turns and wherever concrete butts into vertical surfaces, unless otherwise noted on the Plans.
 - b. Provide waterstops in all construction joints, unless otherwise indicated on the Plans.
 - c. Join all waterstops at all intersections so that a continuous seal is provided. Center the waterstop in the joint. Hold water stop positively in correct position. In the event of damage to the waterstop, repair the water stop in an acceptable manner. Vibrate concrete to obtain impervious concrete in the vicinity of all joints.
 - d. Install waterstop in accordance with instructions of the manufacturer. Prior to use of the waterstop material in the field, submit to the Engineer for approval a sample of each size and shape to be used. Fabricate sample so that the material and workmanship represent in all respects the fittings to be furnished under this Specification.
 - e. Place all sleeves, inserts, anchors, and other embedded items prior to placing concrete. Anchors and bolts cast in concrete shall

be hot dip galvanized or stainless steel. Where permitted by the Engineer, concrete expansion bolts shall be stainless steel and of the wedge anchor type. Take all necessary precautions to prevent embedded items from being displaced, broken or deformed during concreting operation. Protect drains from intrusion of concrete.

- 6. Placing:
 - a. Equipment for mixing and transporting concrete must be clean. Forms shall be thoroughly clean and damp, and reinforcing shall be secured in place. Runaways for transporting concrete shall not rest on reinforcing. When concrete is placed against earth, sprinkle sufficiently before placing.
 - Deposit of concrete in forms no longer than ninety (90) minutes after the initial design water has been added to the cement and aggregates. Concrete which can not be so placed shall not be used and shall be wasted. No additional water shall be added. No retempering with water is permitted.
 - c. In addition to the requirements of ASTM C94, the concrete delivery tickets shall indicate the cement content and water/cement ratio.
 - d. During hot weather, proper attention shall be given to ingredients, production methods, handling, placing, protection and curing. Comply with ACI 305R "Hot Weather Concreting" recommendations.
 - e. Do not place concrete in forms unless the water level is below the concrete to be placed, even if it is necessary to maintain the dewatering, or under rain.
 - f. Do not place concrete under water except for tremie concrete as called for on the Plans. Submit for approval plan and details of means and methods for installation of seal tremie concrete prior to commencement of work. Seal concrete which subsequently fails to perform, shall be repaired or replaced at no additional cost to the Department.
 - g. Place seal concrete under water in the space in which it is to remain, by means of a tremie, a closed-bottom dump bucket of not less than one cubic yard capacity, or other approved method, and do not disturb after it is deposited. Deposit all seal concrete in one continuous pour. Do not place concrete in running water. Design all formwork, to retain concrete under water, to be watertight. Submit shop drawings for the design of formwork and excavation sheeting signed and sealed by a Florida Registered Professional Engineer.
 - h. The tremie shall consist of a tube having a minimum inside diameter of ten (10) inches, and shall be constructed of sections having tight joints. No aluminum parts which have contact with the concrete will be permitted. The discharge end shall be entirely seated at all times and the tremie tube kept full to the bottom of the hopper. When a batch is dumped into the hopper, the tremie shall be slightly raised (but not out of the concrete at the bottom) until the batch discharges to the bottom of the hopper, after which

the flow shall be stopped by lowering the tremie. The means of supporting the tremie shall be such as to permit the free movement of the discharge end over the entire top surface of the work, and shall permit it being lowered rapidly when necessary to choke off or retard the flow. The flow shall preferably be continuous and in no case shall be interrupted until the work is completed. Exercise special care to maintain still water at the point of deposit.

- i. When the concrete is placed by means of a bottom dump bucket, the bucket shall be lowered gradually and carefully until it rests upon the concrete already placed. The bucket shall then be raised very slowly during the discharge travel; the intent being to maintain, as nearly as possible, still water at the point of discharge and to avoid agitating the mixture. Aluminum buckets will not be permitted.
- j. Do not commence pumping, to dewater a sealed cofferdam, until the seal has set sufficiently to withstand the hydrostatic pressure, and in no case earlier than 72 hours after placement of concrete.
- k. Notify Engineer a minimum of 24 hours prior to concreting and request a specific time for observation of reinforcing and formwork for portions of concrete work to be placed. No observation will made by the Engineer until rebar installation for all work to be done and all formwork has been completed and approved by the Contractor's field superintendent. Do not order concrete until all correction and additions indicated by the Engineer have been made. Should the Engineer's observation reveal that work is improperly prepared and an additional observation will be required, he will so inform the Contractor and all above requirements shall also govern.
- 7. Repair of Surface Defects:
 - Repair all concrete surface defects, which includes, but not limited a. to cracks, tie holes (no plastic cones), uneven holes, honey combs, rough frame work and other objectionable conditions deemed unacceptable to the Engineer immediately after form removal. This repair work is to be done for all concrete expose surfaces, liquid applied surface or painted surfaces in or out of the Repair all cracks and defects in the concrete floors, water. beams, joists, columns, and other structural members, roof and walls, to the satisfaction of the Engineer, that may occur up to one year after acceptance of work regardless of the cause. Test unformed, surfaces such as monolithic slabs, for smoothness and verify placement tolerances specified for each surface and finish. Test unformed surfaces sloped to drain for trueness of slope, in addition to smoothness. Repair unformed surfaces that contain surface defects which affect durability of concrete. Surface defects, as such, include cracking, cracks which penetrate to reinforcement or completely through non-reinforced sections regardless of width, spalling, pop-outs, honeycomb, rock pockets and other objectionable and rough conditions.

- b. Proprietary compounds for adhesion or as patching ingredients may be used, if approved by the Engineer. All structural repair of surface defects to be made require the approval of the Engineer, as to the method and procedure. Approval of the completed work must be obtained from the Engineer.
- 8. Finishing of Formed Surfaces.
 - a. Apply rough form finish to exterior walls below grade not exposed to water.
 - b. Apply smooth form finish to exterior and interior walls and columns exposed to water.
 - c. Apply smooth form finish to interior walls and underside of floors, stairs and slabs.
 - d. In addition to the smooth form finish, apply a grout cleaned finish to concrete walls and surfaces exposed to public view and underside of formed floors, stairs or slabs.
 - e. Apply a rubber float grout mix to properly prepared concrete surface, only when approved by the Engineer. Mix shall have one part Portland cement to two parts fine sand in a 50% water and 50% Acryl #60 (Thoroseal or Acryl Set) mix or Acryl Set by Master Builders. Make a 10 x 10-foot sample on the concrete wall for the approval of the Engineer. Finished surface shall be a non dusting hard finish, when scratched with a 1/4-inch metal edge.
 - f. Finish concrete surface, interior or exterior, below or above water shall include all:
 - 1) Exposed concrete.
 - 2) Grout finished concrete.
 - 3) Painted surface concrete.
 - 4) Liquid membrane finished concrete shall comply with manufacturer's requirements.
 - 5) The entire surface of finished concrete shall have a smooth uniform surface, there shall be no offsets, visually bulges, or wavering in the finished surfaces. The joints must be accurately aligned, they can not be uneven or in or out, a higher and lower, there shall be no fins, projection or unevenness between forms.
 - 6) If after stripping the forms the Engineer determines that the finished concrete does not comply with any or all of the above requirements, the Contractor shall submit his proposal in writing to the Engineer as to his methods of correcting the work at no added cost to the Department, which shall include, but not limited to all grinding of fins, projections, unevenness between joints, form high spots and uneven spots.
 - 7) In addition to all other requirements, concrete surfaces exposed to public view, irrespective of size, area or location shall be completely clean and free of: (1) Stains of any nature, (2) Parts of forms or other wood of any nature, (3) laitance, (4) "Run-downs" of leaked water from secondary pours, (5) Nails, (6) Strips, (7) Ties and (8) all other extraneous, deleterious materials and/or substances

which may affect the finished appearance and condition of exposed concrete. Surfaces not meeting the above requirements are to be repaired and treated at no additional cost to the Department.

- 9. Slabs
 - a. Unless otherwise noted on the Plans, place strips alternately at maximum 20 feet center-to-center and to align with column centerline. Do not place adjacent strips until elapse of twenty four hours after first strip is placed. Place slabs on grade by the "strip-cast" method. Method to be reviewed by the Engineer. Provide saw-cut joints at maximum 20 feet center-to-center and to align with column center lines within four hours of final finishing.
 - b. Provide doweled construction joints where shown on the Plans.
 - c. Provide a hard steel troweled finish, free from trowel marks and irregularities, to slabs and floors.
 - d. Provide a light hair-broom finish to exterior slabs and floors exposed to public view. Leave hair-broom lines parallel to direction of the slab drainage.
 - e. Provide a stiff bristle broom finish to slabs and floors with slopes greater than 10 percent. Leave broom lines parallel to slope drainage.
 - f. Finish exposed edges of slabs, floors and tops of walls with a 1/4-inch radius edge unless a chamfer is called for on the Plans.
- 10. Curing and Protection
 - a. Comply with ACI 305 "Hot Weather Concreting", Chapter 4, with the supplements and modifications to ACI 301 listed herein.
 - b. Only concrete water curing for not less than 7 days (24 hours/day continuously) will be accepted; Burlene mats shall be used in curing. Water cure by ponding or continuous sprinkling covering complete surface with minimum runoff. The application of water to wall may be interrupted for grout cleaning only over the areas being cleaned at the time, and the concrete surfaces shall not be permitted to become dry during such interruption.
 - c. Begin all water curing as soon as concrete is set and concrete will not be damaged. Keep concrete and wall forms wet the first 24 hours. Remove forms as indicated in Formwork, Section 3.02-C.4, and continue with 7 day water curing. Recoat damaged surfaces subject to heavy or surfaces damaged by construction procedures within 3 hours of damage. Method of repair shall be approved by the Engineer.
- 11. Testing
 - a. Testing laboratory will be selected and paid for by the Department. Send results of all test to the Department and to the Contractor. The Contractor shall notify the Testing laboratory at least 24 hours before each concrete placing.
 - b. Obtain and mold 3 specimens for each fifty (50) cu. yds., or fraction thereof, of each class of concrete placed each day or as directed by the Engineer.

- c. Cure specimens from each sample in accordance with ASTM C31. Record in test report any deviations from this Standard.
- d. Test specimens in accordance with ASTM C39. Test one specimen at twenty eight (28) days for acceptance and, one specimen at three (3) days and seven (7) days respectively, for information. If one specimen in a test manifests evidence of improper sampling, molding or testing, it shall be discarded and the strength of the remaining cylinders shall be considered the test result.
- e. Contractors Superintendent shall color code on a set of structural drawings the extent of days work and date to conform to cylinders test.
- f. Perform slump test at discharge of mixer, one for each strength test in accordance with ASTM C143. In the event slump is excessive, testing laboratory will immediately notify the Contractor's superintendent and the Engineer's representative on site. The Contractor shall then reject all concrete with excessive slump and/or deposit time.
- g. Drying Shrinkage Test: A drying shrinkage test shall be conducted on the preliminary trial batch with the maximum watercementitious materials ratio used to qualify each proposed concrete mix design using the concrete materials, including admixtures, that are proposed for the project. Three test specimens shall be prepared for each test. Drying shrinkage specimens shall be 4 x 4 x 11 inch prisms with an effective gauge length of 10 inches fabricated, cured, dried, and measured in accordance with ASTM C157 except with the following modifications:
 - Specimens shall be removed from the molds at an age of 1) 23 hours \pm 1 hour after trial batching, shall be placed immediately in water at 73°F ± 3°F for at least 30 minutes, and shall be measured within 30 minutes thereafter to determine original length and then submerged in limesaturated water as specified in ASTM C157. Measurement to determine expansion expressed as a percentage of original length shall be taken at age 7 days. The length at 7 days shall be the base length for drying shrinkage calculations ("0" days drying age). Specimens then shall be stored immediately in a humidity controlled room maintained at 73°F ± 3°F and 50% ± 4% relative humidity for the remainder of the test. Measurements to determine shrinkage expressed as percentage of base length shall be reported separately for 7, 14, and 21 days ±4 hours of drying from "0" day after 7 days of moist curing.
 - 2) Drying shrinkage deformation for each specimen shall be computed as the difference between the base length (at "0" days drying age) and the length after drying at each test age. Results of the shrinkage test shall be reported to the nearest 0.001 percent. If drying shrinkage of any specimen deviates from the average for that test age more than 0.004 percent, the results for that specimen shall be

disregarded.

- 3) The average drying shrinkage of each set of test specimens cast in the laboratory from a trial batch as measured at the 21 days drying age shall not exceed 0.036 percent and 0.042 percent at the 28-day drying stage for all concrete.
 - a) The maximum concrete shrinkage for specimens cast in the field shall not exceed the trial batch maximum shrinkage requirement by more than 25 percent.
 - b) If the required shrinkage limitation is not met during construction, the Contractor shall take any or all of the following actions at no additional cost to the Owner, for securing the specified shrinkage requirements. These actions may include changing aggregates, cement and/or the source or admixtures, including Tetra Guard AS 20 or approved equal; reducing water content; washing of aggregate to reduce fines; increasing the number of construction joints: modifying the curing requirements: or other actions designed to minimize shrinkage or the effects of shrinkage.
- 4) Alkali-aggregate reactivity potential shall be determined in accordance with Appendix XI of ASTM C33. Aggregates shall be tested in accordance with ASTM C289 and C295 to determine potential reactivity. Aggregates which do not indicate a potential for alkali reactivity or reactive constituents may be used without further testing. Aggregates which indicate a potential for alkali reactivity shall be further tested in accordance with ASTM C227 or C1105, as appropriate, using a cement containing less than 0.6 percent alkalies. At the discretion of the Engineer, testing in addition to that indicated in Appendix XI of ASTM C33 may be performed on potentially reactive aggregates. Nonreactive aggregates shall be imported if, in the opinion of the Engineer, local aggregates exhibit unacceptable potential reactivity.
- 12. Evaluation and Acceptance of Concrete.
 - a. If tests are insufficient or inadequate, test and evaluate by core tests. Failure of any concrete cylinder to meet specified requirements shall be deemed as non-complying and costs of additional tests to determine the adequacy or inadequacy shall be borne by the Contractor. Concrete rejected for any reason is to be removed and replaced, including labor, forms and reinforcing, to meet specifications at no additional cost to the Department and no additional time extension.
- 13. Additional Requirements.
 - a. Submit shop drawings as required per General Conditions and

elsewhere in these specifications. Prime Contractor shall check and approve all shop drawings prior to submission. Do not fabricate any item requiring shop drawings until approval of shop drawings has been granted by the Department. Partial shop drawings are not accepted, submit drawings for complete submittal.

- b. Provide precast or cast-in-place reinforced concrete lintels at all masonry openings and sills at all windows. Reinforce to suit loads and span. Provide minimum 8-inch bearing at each end and, pour integral with columns where opening abuts columns.
- c. Sidewalks in R.O.W.: Provide poured-in-place 4-inch thick concrete slab, 3,000 psi concrete, with continuous 8-inch deep thickened slab edges. Isolate walks from vertical surfaces with 1/2-inch expansion joint material. Provide 1/2-inch expansion bituminous joint material flush with top of concrete slabs at 20 feet on center and tooled joints at 5 feet on center. Tool all open edges to a smooth radius and all edges adjacent to the forms.

END OF SECTION

APPENDIX "A"

MIAMI-DADE COUNTY DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES DIVISION OF ENVIRONMENTAL RESOURCES MANAGEMENT

NOTICE TO ALL CONTRACTORS

(1 Page)

MIAMI DADE COUNTY, FLORIDA DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES 701 NW 1ST COURT SUITE 200 MIAMI, FLORIDA 33136 (305) 372-6789

NOTICE TO ALL CONTRACTORS INVOLVED IN ANY CONSTRUCTION ACTIVITY WHICH REQUIRES DEWATERING WITH ULTIMATE DISCHARGE INTO A CANAL, LAKE, DITCH OR STORM SEWER WHICH DISCHARGES INTO AN OPEN BODY OF WATER OR BISCAYNE BAY.

Please be aware that if you are involved in any construction activity as above described, you are required to provide all necessary measures in order to maintain turbidity in the receiving body of water within the acceptable limits as established by the Florida Building Code. You must present a separate plan to be included with your building plans indicating your proposed measures or apply for a permit from Miami-Dade County Department of Regulatory and Economic Resources before your construction plans will receive final approval. For additional information, please contact Maria Molina, P.E. Chief, Water Control Section at 305-372-6769.

APPENDIX "B"

MIAMI-DADE WATER AND SEWER DEPARTMENT

STANDARD DETAILS

(45 Pages)

- 1. ALL MATERIALS AND LABOR UNDER THIS PROJECT SHALL BE IN STRICT ACCORDANCE WITH THE REQUIREMENTS OF THE MIAMI-DADE WATER AND SEWER DEPARTMENT AND SHALL CONFORM TO THE STANDARDS AND SPECIFICATIONS AVAILABLE AND ON FILE WITH THE DEPARTMENT. SUBMIT SHOP DRAWINGS FOR ALL MATERIALS.
- 2. COVER OVER WATER OR SEWER FORCE MAINS SHALL BE 4'-0" MIN.
- 3. ALL MAIN LINE VALVES SHALL BE INSTALLED COMPLETE WITH 10" RISER PIPES AND NO. 3 OR 53 VALVE BOXES FIRE HYDRANTS AND SERVICE VALVES SHALL BE INSTALLED COMPLETE WITH 6" RISER PIPES AND NO. 2 VALVE BOXES.
- 4. ALL FORCE MAIN SERVICE CONNECTIONS INTO PRESSURE TRANSMISSION MAINS SHALL HAVE A SHUT OFF VALVE AND CHECK VALVE AT THE POINT OF ENTRY.
- 5. ALL GRAVITY SYSTEMS SHALL BE CONSTRUCTED IN ACCORDANCE WITH DEPARTMENT STANDARDS.
- 6. ALL WATER METERS WILL BE INSTALLED BY THE MIAMI-DADE WATER AND SEWER DEPARTMENT, PROVIDING THE APPROPRIATE CHARGES HAVE BEEN PREPAID.
- 7. FIRE HYDRANT REQUIREMENTS (NUMBER AND LOCATION) SHALL BE AS REQUIRED BY MIAMI-DADE COUNTY FIRE DEPARTMENT OR THE APPROPRIATE FIRE AGENCY WITH INSTALLATION IN ACCORDANCE WITH DEPARTMENT STANDARDS.
- 8. CONTRACTOR MUST CALL MDWASD INSPECTION DIVISION TO ARRANGE FOR A PRECONSTRUCTION MEETING 2 FULL BUSINESS DAYS PRIOR TO PROPOSED START OF CONSTRUCTION. CONTACT ONE CALL CENTER 48 HRS PRIOR TO EXCAVATION.
- 9. CONTRACT INSPECTOR WILL INSPECT ANY FACILITIES APPROVED BY THE DEPARTMENT. ALL OTHER REQUIREMENTS OF THE PERMITTING AGENCY SHALL BE IN ACCORDANCE WITH THEIR STANDARDS AND REQUIREMENTS.
- 10. WORK PERFORMED UNDER THIS PROJECT WILL NOT BE CONSIDERED AS COMPLETE UNTIL FINAL ACCEPTANCE OF THE SYSTEM BY THE DEPARTMENT AND UNTIL THE FOLLOWING DOCUMENTS ARE RECEIVED AND APPROVED BY THE DEPARTMENT:
- a. EASEMENTS, IF REQUIRED
- b. CONTRACTOR'S WAIVER AND RELEASE OF LIEN
- c. ABSOLUTE BILL OF SALE
- d. i. CONTRACTOR'S LETTER OF WARRANTY (I.E., LETTER AGREEMENT) ii DEVELOPER'S CONTRACT BOND (I.E., CONTRACT AGREEMENT).
- e. "RECORD DRAWING" PRINTS (24"x 36") SHOWING SPECIFIC LOCATIONS, DEPTH, ETC. OF ALL WATER AND SEWER FACILITIES AS LOCATED BY A LICENSED SURVEYOR & MAPPER, ALONG WITH PRINTS OF "RECORD DRAWINGS" WHICH HAVE BEEN SIGNED AND SEALED BY A REGISTERED SURVEYOR & MAPPER. (No. OF PRINTS: 3-FOR WATER, 4-FOR GRAVITY SEWER AND 5-FOR FORCE MAIN OR PUMP STATION PROJECTS). Submittal of final CAD Files required.
- f. H.R.S. LETTER OF RELEASE REQUIRED FOR ALL WATER PROJECTS
- g. BILL OF SALE SKETCH (8½"x 11") FOR WATER AND SEWER, SEPARATELY
- 11. ALL NEW CONNECTIONS FROM EXISTING DEPARTMENT MAINS TO BE MADE BY DEPARTMENT FORCES ONLY. THE CONTRACTOR TO EXCAVATE AT REQUIRED LOCATIONS, PROVIDE AND INSTALL MATERIAL WITH FITTINGS, PRIOR TO TAP.
- 12. AN APPROVED PAVING AND DRAINAGE PLAN MUST BE SUBMITTED TO MDWASD FOR ALL NEW SUBDIVISIONS PRIOR TO APPROVAL OF WATER AND SEWER PERMIT PLANS, UPON REQUEST.

13. UNLESS OTHERWISE SPECIFIED, ALL TAPS 20 INCHES AND SMALLER FOR CONNECTIONS TO EXISTING MAINS WILL BE DONE BY DEPARTMENT FORCES. UNDER NO CIRCUMSTANCES WILL THE CONTRACTOR BE PERMITTED TO TAP EXISTING MAINS IN THE SIZE RANGE SPECIFIED ABOVE. THE TAPPING SLEEVE AND TAPPING VALVE ARE FURNISHED AND INSTALLED BY THE CONTRACTOR UNDER THE SUPERVISION OF THE INSPECTOR

UNDER THE S	SUPERVISION OF TH	HE INSPECTOR.						
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WATER & SEWER DEPARTMENT					JILLI			

1. AT THE COMPLETION OF ANY WATER AND SEWER JOB EITHER DONATION OR CONTRACT, THE CONTRACTOR SHALL SUBMIT:

a. RECORD DRAWING PRINTS WHICH HAVE BEEN SIGNED AND SEALED BY A FLORIDA LICENSED PROFESSIONAL SURVEYOR AND MAPPER (QTY. OF PRINTS AS REQUIRED BY THE DEPARTMENT).

- 2. "RECORD DRAWING" FORMAT:
 - a. 24"x 36" PRINTS
 - b. PDF FILE
 - c. CADD FILE (DWG OR DXF) ROTATED AND TRANSLATED TO STATE PLANE COORDINATES NAD 83 FLORIDA EAST ZONE
 - d. THE WORDS "RECORD DRAWING" IN LARGE LETTERS
 - e. TITLE BLOCK WITH DEPARTMENT DS, DW OR ER NUMBER AND PERTINENT INFORMATION
 - f. PREFERRED SCALE TO BE 1"= 40' HORIZONTALLY AND 1"= 4' VERTICALLY*
 - g. STREET NOMENCLATURE
 - h. SEPARATE RECORD DRAWINGS FOR WATER AND SEWER
 - i. SEPARATE WATER AND SEWER PROFILE
 - j. STATIONING STARTING WITH 0+00 AT PERMANENT REFERENCE POINT (I.E. €, §, ETC.) OR AS SHOWN ON DESIGN PERMIT PLANS, AND TO RUN CONTINUOUSLY TO END OF MAIN
 - k. EASEMENTS, IF ANY, TIED TO PERMANENT REFERENCE POINT
 - I. IDENTIFY ALL CONTROL LINES (I.E. BLDG. LINE, PROPERTY LINE, R/W, ETC.)
 - m. ALL "PROPOSED" INFORMATION TO BE REMOVED FROM PRINTS, LEAVING ONLY RECORD DRAWING INFORMATION REFLECTED IN DRAWINGS
- 3. WATER "RECORD DRAWINGS" MUST INCLUDE:
 - a. PLANS SHOWING PIPE SIZE, MATERIAL AND OFFSET OF MAIN, DEFLECTIONS (IF ANY), STATION OF SERVICES, HYDRANTS, VALVES, FITTINGS, IF ANY, ALL IN STATE PLANE COORDINATES. UTILITY CROSSINGS SHALL BE CLEARLY IDENTIFIED AND LOCATED.
 - b. PROFILE SHOWING TOP OF GROUND AND TOP OF PIPE ELEVATIONS AT EVERY 100' STATION AND AT ANY CHANGE IN GRADE (WITH CORRESPONDING STATION), PIPE SIZE AND PIPE MATERIALS REFERENCED TO PLAN.
- 4. SEWER "RECORD DRAWINGS" MUST INCLUDE:
 - a. PLAN SHOWING MANHOLE NUMBER, PIPE SIZE AND PIPE MATERIAL OF PIPE, DEFLECTION, SLOPE OF GRAVITY SEWER, LOCATION OF LATERALS WITH REFERENCE TO MANHOLE AND CLEANOUTS.
 - b. THE NORTHERLY AND EASTERLY COORDINATES ON ALL FIELD OBTAINED MEASUREMENTS AND PROVIDED ON ALL RECORD DRAWING SUBMITTALS
 - c. PROFILE SHOWING MANHOLE NUMBER (AS PER PLAN), RIM AND INVERT ELEVATIONS (IF MORE THAN ONE INVERT, LABEL NORTH, SOUTH, ETC.), AND STATION STARTING AT 0+00 AT DOWNSTREAM MANHOLE.
- 5. FORCE MAIN "RECORD DRAWING" SAME AS WATER MAIN.
- 6. EACH RECORD DRAWING SHALL SHOW THE FLORIDA STATE PLANE COORDINATES (CURRENT READJUSTMENT) OF ALL THE MANHOLES AND VALVES AND OF AT LEAST TWO HORIZONTAL CONTROL POINTS PROPERLY IDENTIFIED AND LOCATED WITHIN THE PROJECT.

* OTHER SCALE	MAY BE PERMITTE	D BUT MUST BE	APPROVED				
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NOTES:

1.	STORM SEWER, PROVIDE A MIN OF THE LOWER THE STORM/W. OF CROSSING VERTICAL SEPA CROSSING OVE ARRANGEMENT, IS NOT REQUIP	, GRAVITY WASTEWAT NIMUM VERTICAL DIS R PIPE. WHERE THIS ASTEWATER/RECLAIM WITH NO LESS THA ARATION SHALL BE IR A POTABLE WATE AS STATED ABOVE RED FOR STORM SE	TER AND RECLAIMED TANCE OF TWELVE (S MINIMUM SEPARATI IED WATER PIPE JOI N TEN (10) FEET B 6 INCHES. WHERE T R MAIN, THE CRITEF , SHALL BE REQUIRI WERS.	WATER MAIN CROSSING UNDER (12) INCHES BETWEEN THE INVE ON CANNOT BE MAINTAINED, TH NTS AND POTABLE WATER MAIN ETWEEN ANY TWO JOINTS, BOTH HERE IS NO ALTERNATIVE TO S RIA FOR MINIMUM 12" VERTICAL ED, AND BOTH PIPES SHALL BE	POTABLE WATER MAINS SHALL ERT OF THE UPPER PIPE AND E CROSSING SHALL BE ARRAND JOINTS ARE EQUIDISTANT FROM I PIPES SHALL BE D.I.P., AND TORM/WASTEWATER/RECLAIMED SEPARATION BETWEEN LINES A D.I.P. IRRESPECTIVE OF SEPAR	BE LAID T THE CROWI SED SO TH I THE POIN THE MINIM WATER PIP ND JOINT RATION. D.I	ro N AT VT UM ES .P.
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	45° HORIZONTAL BEND		6	8	11	13	15	19	22	26	30	35	38	42		
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		BEND	10	14	19	23	28	35	43	50	59	67	75	82		
	22.5° VERTICAL	UPPER BEND	7	12	17	21	26	34	42	49	60	70	78	87		
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<u>LEGEND</u>

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ELECTRIC CABLE UNDREGROUND (U) OVERHEAD (O)	SIZE ?_ELEC
ELECTRICAL PULL BOX	SIZE ? ELEC.
STORM SEWER	ØSS
CATCH BASIN OR INLET	C.B.



<u>LEGEND</u>

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PROPERTY LINE	──₽─ ─ ─
Center Line of Pavement	<u> </u>
BASE LINE	
EDGE OF PAVEMENT LINE	

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Delivering Excellence Every Day WATER & SEWER DEPARTMENT						SHEET 2	2 OF 2





THE FOLLOWING REQUIREMENTS SHALL APPLY TO ALL CASTINGS (OR FABRICATED MATERIALS) CONTAINED HEREIN:

- 1. SUB-COMPONENTS OF ALL CASTINGS FROM THE SAME SUPPLIER SHALL BE INTERCHANGEABLE.
- 2. MATING SURFACES OF ALL CASTINGS SHALL BE MACHINED, WITH NO ROCKING PERMITTED.
- 3. ALL CASTINGS SHALL BE IN ACCORDANCE WITH ASTM-A48, AS MODIFIED HEREIN:
- A. ARTICLE 10.1.3.1. SHALL NOT APPLY, TIME LIMIT IS 4 HRS. MAXIMUM.
- B. NOTIFICATION TIME LIMIT IN ARTICLE 16.2 SHALL NOT APPLY.
- C. SUPPLIER SHALL PROVIDE CERTIFIED TEST RESULTS WITH EACH LOT OF CASTING SHIPPED. CERTIFICATION SHALL IDENTIFY LOT.
- D. SIGNATURE IN ARTICLE 17.2 SHALL BE REQUIRED. CERTIFICATION SHALL BE SIGNED BY LICENSED PROFESSIONAL ENGINEER, OR EQUIVALENT WHEN IN FOREIGN COUNTRY.
- E. SUPPLIER SHALL PROVIDE CAST TEST BAR, SUITABLE FOR MACHINING, FOR EACH FOUNDRY LOT OF CASTINGS SHIPPED. TEST BAR SHALL IDENTIFY LOT AND SHALL BE SUITABLE FOR CLASS NO. 35B SPECIMEN.
- 4. MANUFATURER'S NAME AND LOCATION (I.E. FOUNDRY AND COUNTRY OF ORIGIN) SHALL BE PERMANENTLY CAST ON THE BOTTOM OF ALL CAST COVERS AND LIDS.
- 5. SUPPLIER OF FABRICATED ITEMS (I.E. NON-CAST ITEMS) SHALL PERMANENTLY MARK HIS NAME AND DATE OF MANUFACTURE ON MATERIAL, BY WELDING, STAMPING OR OTHER METHOD APPROVED BY THE DEPARTMENT.

				ITEM	CROSS	SPEC.
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	<u>ISSUE DATE</u>	<u>APPROVED BY</u>	STANDARI	DETAIL	/۸۸	2
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Method "A"

Cut the polyethylene tube 2 ft. longer than the length of pipe section. Slip the tube around the pipe so as to allow 1 ft. overlap at each end. Overlap the other pipe section after pipe is installed.

Method "B"

Cut the polyethylene tube 1 ft. shorter than the length of pipe sections. Slip the tube around the pipe so as to allow 6 in. of bare pipe at each end. Before making a joint, slip a 3 ft. Length of polyethylene tube over the preceding pipe section. Overlap by at least 1 ft. and secure, after joint is made.

Method "C"

Wrap odd shaped fitting with sheet or split length of polyethylene tube by passing the sheet under the fitting and bringing it up around the body. Make seams by bringing it folding over twice, and tapping down. Tape sheet securely in place at valve stem and other penetrations.

NOTES:

- 1. The Department reserves the right to require polyethylene encasement wherever, in their opinion, corrosive soils exist.
- 2. Tube size will be as listed in table.
- 3. Pipe-shape fittings (bends, reducers, etc.) shall be treated according to Methods "A" and "B". Odd shaped fittings (valves, tees, etc.) shall be treated according to Method "C".
- 4. 6" adhesive tape shall be used to secure encasement.
- 5. Special care shall be taken to prevent damage to wrapping when placing backfill.
- 6. Refer to ASTM D1248 for approved material and accessories.
- 7. Only virgin polyethylene material having a minimum thickness of 8 mils is approved.



		ADAP. A.R.V. ASPH. A.C.P. Ave. alpha B.J. B. B.O. BOT. BUTT. V. alpha C.V. COR. C.B. C.C. C.I.P. CONN. CONC. COND. CONT. C.B. # C.M.P. CORP. STOP CU. YD. DET. D.I.P. DWG. DIV. DWY. E. EL. EL. EL. EL. EL. EL. EL.	Adapter Air Release Valve Asphalt Asbestos Cement Pip Avenue Base Line Break Joint Bell Blow-Off Bottom Butterfly Valve Center Line Check Valve Corner Catch Basin Center to Center Cast Iron Pipe Connection Concrete Conduit Contract Core Boring <i>#</i> Corrugated Metal Pip Corporation Stop Cubic Yard Detail Ductile Iron Pipe Drawing Division Driveway East Elevation Electric Edge of Pavement Existing Full Bevel Field Book No. Fire Hydrant Flanged Outlet Footing Force Main Flushing Valve Outlet Galvanized Galvinized Iron Pipe Gas Gate Valve Gauge	e e	CROSS SPEC. RFF RFF
MIAMI-DADE COUNTY Delivering Excellence Every Day WATER & SEWER DEPARTMENT	<u>ISSUE_DATE</u> 06/01/2008	<u>APPROVED BY</u> V.F.C.	STANDARD ABBREVIAT DEPARTMENT'	DETAIL IONS FOR S PROJECTS	A 10.0 SHEET 1 OF 3

MIAMI-DADE COUNTY Defivering Excellence Every Day WATER & SEWER DEPARTMENT	<u>ISSUE_DATE</u> 06/01/2008	<u>APPROVED BY</u> V.F.C.	ITEM STANDARD DETAIL ABBREVIATIONS FOR DEPARTMENT'S PROJECTS	CROSS REF. REF. A 10.0 SHEET 2 OF 3
		H.B.J. H.L. H.B. H.F.B. H.H.B. INT. INV. LAT. LGTH. L.L. L.P. LT. LOC. M.B. MH. MIN. MAX. M.J. MIP. N. No. PVMT. P.E. P.V. PSI P.C. P.C. P.C. P.C. P.I. LBS. P.O. PROP. P.V.A. RT. R/W R.C.P. REQ'D. REST. REV. SH. S. SQ. FT. S.	Horizontal Break Joint Horizontal length Half Bevel Horizontal Full Bevel Interceptor Invert Lateral Length Laid Length Light Pole Left Location Mail Box Manhole Minimum Maximum Maximum Maximum Mechanical Joint Monument Line Nipple North Number Pavement Plain End Plug Valve Pounds per Square Inch Property Line Point of Curvature Prestressed Concrete Cylinder Pipe Point of Intersection Pounds Push-on Proposed Polyvinyl Chloride Right Right of Way Reinforced Concrete Pipe Required Restrained Range Revised-Revision Short Section Line South Square Feet Spigot	

		SLV. STL. P. ST. SAN. S.S. STA. SPK. TAP. SLV. TAP. V. TEL. TEL. BOX T.F.O. T.L. T.O.P. TRANS. P. TWP. TYP. T.R. U.P. UNK. V.C.P. W. W.M. W/O W. MTR. X.	Sleeve Steel Pipe Street Sanitary Sewer Storm Sewer Station Sprinkler Tapping Sleeve Tapping Valve Telephone Telephone Box Tapping Flanged Outlet Traffic Light Top of Pipe Transite Pipe Township Typical Thrust Resistant Utility Pole Unknown Vitrified Clay Pipe West Water Main Without Water Meter Extra		
	ISSUE DATE	<u>APPROVED BY</u>	I T E M S T A N D A R D D E T A I L	CROSS SPEC. REF. REF.	
	<u>ISSUE_DATE</u> 06/01/2008	<u>APPROVED BY</u> V.F.C.	ITEM STANDARD DETAIL	CROSS REF. REF. A	
	<u>ISSUE DATE</u> 06/01/2008	<u>APPROVED BY</u> V.F.C.	ITEM STANDARD DETAIL ABBREVIATIONS FOR	CROSS SPEC. REF. REF.	
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		¥¥.IVI.			
		W M	Water Main		
		Ψ.	West		
		V.O.I .			
		V.C.P.	Vitrified Clav Pipe		
		LINK	Unknown		
		0.1.			
		U.P	Utility Pole		
		1.11.			
		T.R.	Thrust Resistant		
		TYP.	Typical		
		1 III . Tr (D			
		TWP	Township		
		TIVANO. F.			
		TRANS P	Transite Pipe		
		1.0.2.	TOP OF PIPE		
		ТОР	Top of Pine		
		I.L.	Traffic Light		
		Τι	Traffic Light		
		I.F.U.	iapping Flangea Uutlet		
		ΤΕΛ	Tapping Flanged Outlat		
		IEL, BUX	leiepnone Box		
		TEL BOX	Telephone Box		
		TEL ROX	Telephone Box		
			Telephone Box		
		ILL.			
		TEI	Telephone		
		TAP. V.	Tapping Valve		
		TAP. SLV.	Tapping Sleeve		
		SPK.	Sprinkler		
		STA.	Station		
		J.J.			
		22	Storm Sewer		
		SAN.	Sanitary Sewer		
		CAN	Capitary Cower		
		ST.	Street		
		SIL. P.	Steel Pipe		
1			Steel Dine		
1		JLV.	Sleeve		
		ST/V			
		SITI. CLV	SHEEL		
		SHT.	Sheet		
		SEC. SHT.	Section Sheet		

APPENDIX "C"

MIAMI-DADE WATER AND SEWER DEPARTMENT

WASD ID CARD SECURITY PROCEDURES NON-WASD EMPLOYEES

(Not Used)

APPENDIX "D"

MIAMI-DADE COUNTY CODE SECTION 2-8.4. PROTEST PROCEDURES

MIAMI-DADE COUNTY ADMINISTRATIVE ORDER NO. 3-21 BID PROTEST PROCEDURES

(Not Used)

APPENDIX "E"

QUARTERLY REPORTS

(2 Pages)

QUARTERLY REPORTS

(MIAMI-DADE COUNTY CONTRACT WORK)

	(name of Count	ty Contractor),
pursuant to County Resolution R-1634	-93, has for the (1st, 2nd, 3rd c	or 4th) Quarter
of 20, received the following	sum from Miami-Dade County on all Miami	-Dade County
projects or contracts the Contractor ha	d during the Quarter.	
\$	dollars and	cents.
Bv:		
Signature	e of Representative of Contractor	
Printed Nar	me of Representative of Contractor	
Capacity of Rep	resentative, if not a Sole Proprietorship	
	Address of Contractor	

QUARTERLY REPORTS

(PRIVATE SECTOR WORK)

		(name of County Contractor),
pursuant to County Resolution R-163	34-93, has for the	(1st, 2nd, 3rd or 4th) Quarter
of 20, received the following	g sum on Private Sector Work	the Contractor had during the
Quarter.		
\$	dollars a	and cents.
Ву:		
Signat	ure of Representative of Contractor	
Printed N	Name of Representative of Contractor	
Capacity of R	Representative, if not a Sole Proprieto	rship
	Address of Contractor	

QUARTERLY REPORTS PRIVATE SECTOR WORK Page 1 of 1